



Thesis Proposal

Army National Guard Readiness Center
Arlington, Virginia

Prepared By: Amanda C. Farace | Structural Option
Prepared For: Dr. Thomas E. Boothby
December 11, 2009

Table of Contents

| | |
|--|-----------|
| EXECUTIVE SUMMARY | 3 |
| INTRODUCTION | 4 |
| BACKGROUND | 5 |
| EXISTING STRUCTURAL SYSTEM..... | 6 |
| FOUNDATION | 6 |
| COLUMNS..... | 6 |
| FLOOR SYSTEMS | 7 |
| ROOF SYSTEMS..... | 8 |
| LATERAL SYSTEM..... | 8 |
| PROPOSED STATEMENT | 9 |
| METHODOLOGIES & SOLUTIONS..... | 9 |
| BREADTH TOPIC 1 – ARCHITECTURE..... | 10 |
| BREADTH TOPIC 2 – CONSTRUCTION MANAGEMENT | 10 |
| TASKS & TOOLS | 11 |
| SCHEDULE..... | 13 |
| CONCLUSION..... | 14 |

EXECUTIVE SUMMARY

Depth Study: Steel Re-design

The Army National Guard Readiness Center addition is an 8-story joint headquarters building located in Arlington, Virginia. It consists of 3 underground levels, 5 above grade levels, and a mechanical penthouse. The building is constructed of cast-in-place concrete with reinforced shear walls. The building includes large data areas, a library fitness center, open office space and office suites, and sensitive areas protected by SCIFS. A large plaza at the 1T level doubles as a green roof. It is anticipated to achieve a LEED silver rating.

Due to the national significance of the building, the structural design of the Army National Guard Readiness Center Addition was guided by progressive collapse and blast requirements. For the purpose of this thesis, the problem of progressive collapse shall be reevaluated for an alternate steel framing system. With the added risk of terrorist attacks and other threats on the building's structure, blast and explosion loading will also be considered for the new steel structure. Through research and analysis of various load cases and various collapse scenarios, the structure will be designed to protect the occupants from potential disasters.

Breadth 1: Architectural

The change in structural systems from concrete to steel will inevitably have an effect on the floor layouts. After the steel structure is redesigned and it becomes clear where it will impact the floor plans, spatial relation exercises such as bubble diagrams will be used to reconfigure the plans and create a working typical floor plan that fits with the proposed structure. Final drawings will be created using AutoCAD and will be included in the final report. Along with the floor plans, the building's façade will also be affected by the change in structural layout. Sketches will be used to determine a viable solution to the façade.

Breadth 2: Construction Management

Curing time and formwork construction can be eliminated from the construction schedule when changing the structural system from concrete to steel. Procurement time however, may increase the construction time and must be considered as well. To account for this and other impacts the redesign will have on the project schedule and cost, an in-depth analysis will be completed to determine if the proposed steel system is an economical alternative to the existing concrete structure. To ensure that this analysis is as accurate and true to life as possible, general contractors, sub-contractors and vendors from the Arlington areal will be consulted.

INTRODUCTION

The Army National Guard Readiness Center headquarters addition sited to the south of the existing facility, where storm water retention ponds were previously located. Due to the loss of the retention pond, the project also includes the installation of storm water detention tanks. The new building is 82 feet above grade and approximately 251,000 square feet. The contract value was \$100 million and is a Design-Bid-Build with Tompkins Builders, Inc., the general contractor, holding lump sum contracts with all subcontractors. The eight-story facility is comprised of 3 underground levels (Referred to as Levels 3P, 2P and 1P) and a 5 level tower component (Levels referred to as 1T – 5T) as well as a mechanical penthouse. The three underground levels account for the majority of the building's square footage, with a much larger footprint than the above ground floors. The underground encompasses approximately 150,000 square feet and the five-story tower encompasses 100,000 square feet. This design was developed to increase the amount of green space since a large portion of the underground levels will be topped with an intensive green roof system.

The addition is designed to meet Department of Defense Anti-Terrorism and Force Protection Requirements. This required that physical security measures, such as internal bracing to prevent progressive collapse, blast walls, berms, bollards and heavy landscape, to have been integrated into the design of the building. The facility is also expected to achieve LEED Silver Certification. LEED points are anticipated through the green roof system, offering bicycle storage and changing rooms, low-emitting and fuel efficient vehicles, reduction of water usage, water efficient landscaping, use of low-emitting as well as recycled and regional materials, and creating office space that can be 75% daylight. The building will incorporate open office spaces, general office suites, conference rooms, specialized compartmented information facilities, a fitness center, small library, and an auditorium.

As a result of the location and the existing facilities that are on site, several other features have been incorporated into the project. This includes the installation of the storm water detention tanks, the relocation of an existing radio tower, relocation of existing gate, a one story bridge connecting the new facility with the existing headquarters, construction of a new mailroom, and a construction of a new multi-story parking facility. This report will focus on the new Army National Guard Readiness Center Addition and none of the other project features will be discussed or analyzed.

BACKGROUND

The Army National Guard (ArNG) Readiness Center is located at 111 South George Mason Drive in Arlington County, Virginia. The site is bordered on the east by the U.S. Department of State, National Foreign Affairs Training Center, on the north by Arlington Boulevard, on the west by George Mason Drive, and on the south by a residential community. The fifteen-acre site is comprised of a 248,000 square foot headquarters facility, two 3-story parking garages and several small out buildings.

The Army National Guard Readiness Center houses administrative and resource functions that provide support and liaison to the National Guard in all 50 states and requisite territories and to the Pentagon. Currently there is about 1,300 staff based at this facility. The 2005 Base Realignment and Closure Act (BRAC) actions required the realignment of Jefferson Plaza 1 in Crystal City by relocating National Guard Bureau Headquarters and Air Force Headquarters to the Army National Guard Readiness Center in Arlington and to Andrews Air Force Base, in Maryland. This means the relocation of more than 1,200 National Guard Bureau Joint Staff and Army National Guard Staff to relocate to the Readiness Center. This relocation has created a great need for a Readiness Center Addition. Due to the BRAC Requirements the 1,200 personnel must be relocated before 2011. This makes the construction schedule particularly crucial.



Figure 1: West Perspective

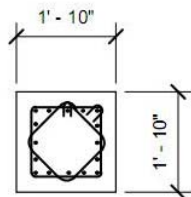
EXISTING STRUCTURAL SYSTEM

Foundation

The geotechnical report engineering survey was performed by CH2M Hill on April 21, 2008. In this study, it was found that a relatively high water level of approximately 6 feet to 10 feet below the existing surface was anticipated. As much as 35 feet of excavation were required to reach the building grades therefore, drilled in soldier piles with wood lagging and tied-back anchors was recommended for temporary excavation support as well as the installation of dewatering well points. CH2M Hill noted that, with proper ground water management and control, the existing subsurface is suitable for support of the building using a mat foundation system based on evaluation of allowable bearing capacity and anticipated settlement. The recommended allowable bearing capacity for the new building location was 4800 lbs/ft² for a mat footing. As a result, a 43-inch concrete mat foundation was designed.

Columns

A reasonably consistent column layout exists throughout the building even with the changes in the shape of the floors between level 3P and 1T. The typical interior gravity column is a 22-inch by 22-inch, reinforced normal weight concrete column. The strength of all columns is 4,000 pounds per square inch. While the size and shape of the column is the same on each floor, there are three changes in reinforcement. For levels 3P to 1P columns are reinforced with sixteen No. 10 vertical bars. These change after the 1P level where the tower component of the building begins. For levels 1T and 2T columns are reinforced with sixteen #8 vertical bars. The reinforcement changes again at the 3T level up to the 5T level; these columns are reinforced with eight #8 vertical bars. #3 ties are located 12 inches on center at every level.



(16) #8 VERT. BARS W/
#3 TIES @ 12" OC

Figure 2: Typical Interior Column

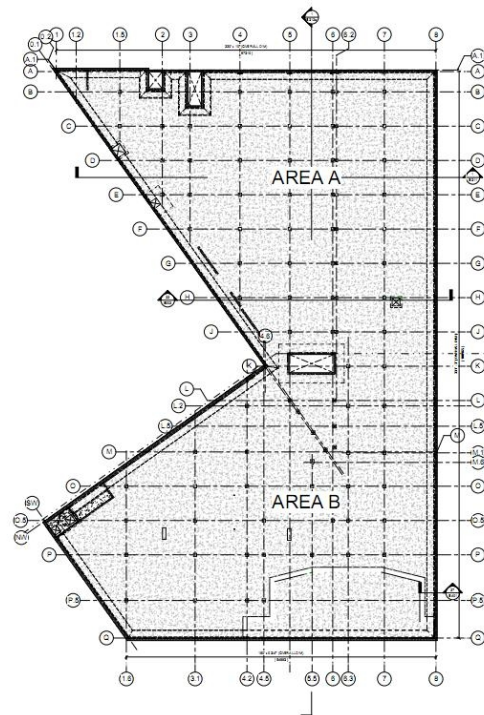


Figure 3: Typical Column Layout for Underground Levels

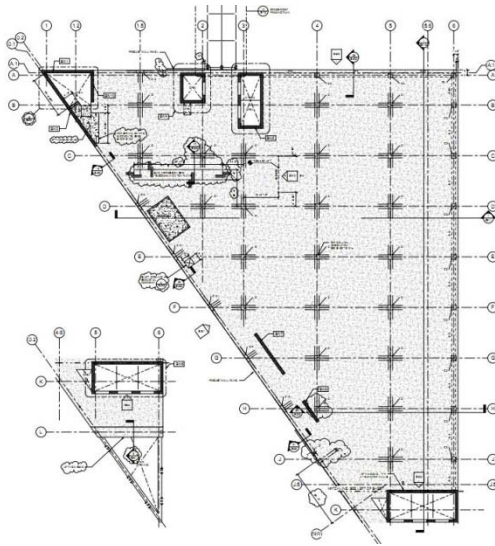


Figure 4: Typical Column Layout for Tower Levels

Floor Systems

The Army National Guard Readiness Center Addition utilizes a reinforced concrete structural system. All of the floors are two-way flat slab with column strips and edge beams along the eastern and northern walls of the Tower component. The typical concrete strength is 4,000 psi. The typical slab thickness is nine inches however; this changes in areas where the access flooring changes and for drainage areas in mechanical and electrical rooms. No. 6 and No. 8

bars are typically used for reinforcement in the floor systems.

Due to the irregular shape of the building and the change in shape from the underground portion of the building to the tower component, a two-inch expansion joint is located at the 3P to 1T levels along column line 6.2. This expansion joint makes the building act as almost two separate buildings, the tower portion and the plaza portion.

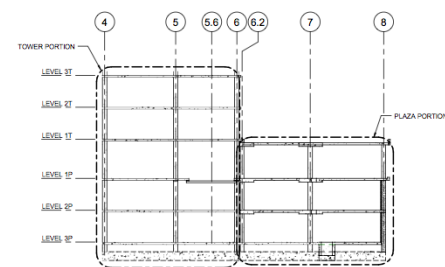


Figure 6: Elevation showing location of expansion joint and relationship between Plaza portion and Tower portion

at the 3P to 1T levels along column line 6.2. This expansion joint makes the building act as almost two separate buildings, the tower portion and the plaza portion. The tower portion extends from level 3P to 5T while the plaza portion is comprised of the subgrade levels and topped of with an intensive green roof. This can be seen in figures 8 and 9.

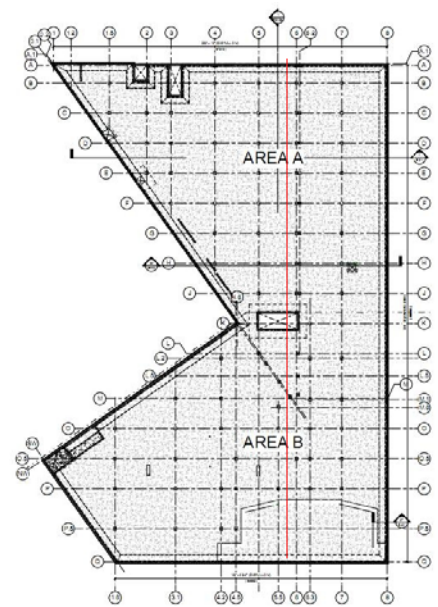


Figure 5: Location of Expansion Joint

Roof Systems

The penthouse roof of the tower is a two-way flat slab. The slab is 10" thick with a concrete strength of 4,000 pounds per square inch. This roof was designed to hold a 30 pounds per square foot snow load and is reinforced with #5 bars at 12 inches on center and 18 inches on center. A large skylight over the northern stairs required steel framing, which consists of beams ranging from W12x14 to W12x26.

The plaza roof is also a two-way slab with drop panels. The slab thickness ranges from eight inches to sixteen inches with a concrete strength of 4,000 pounds per square inch. This roof will act as an intensive green roof and therefore had to be designed to carry a 100-pound per square foot roof garden load. It is reinforced with #6 bars and includes a two-inch expansion joint where the roof abuts the floor of the first tower level (1T), as do the floors below.

Lateral System

The lateral system for the ArNG Readiness Center consists of reinforced concrete shear walls. These walls have a thickness of twelve inches and a concrete strength of 4,500 pounds per square inch. The numbers of shear walls varies between levels due to the building's change in footprint. Typical shear wall locations can be seen in figures 10 and 11 below. This system resists lateral loads in the north-south and east-west direction depending upon the orientation of the wall.

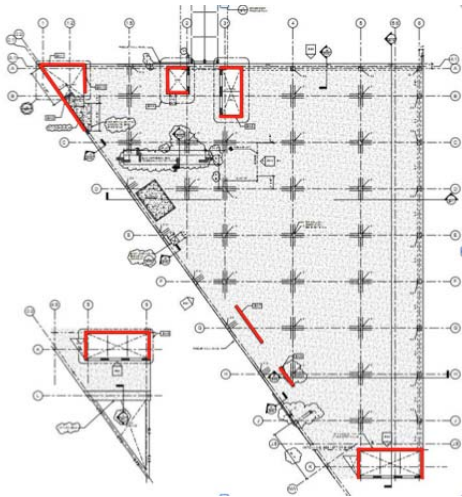


Figure 7: Typical Shear Wall Locations in Below Grade Levels

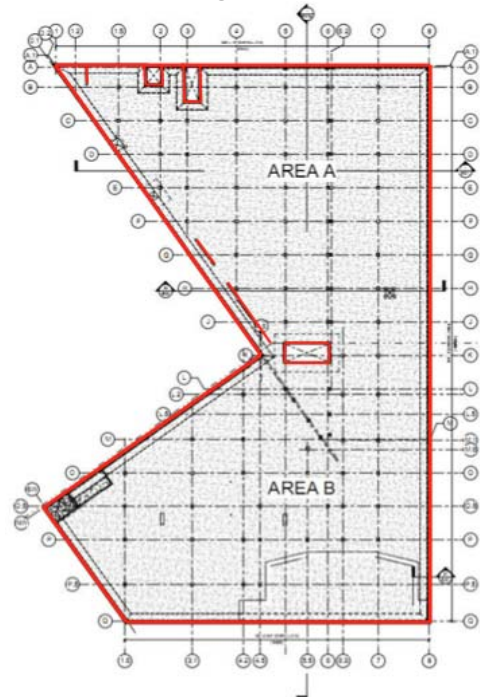


Figure 8: Typical Shear Wall Locations in Tower Levels

PROPOSED STATEMENT

The Army National Guard Readiness Center was designed as a cast-in-place concrete building with two-way flat slab floors and normally reinforced concrete shear walls. From the analysis in previous technical reports, it was confirmed that the structural elements were sufficiently designed to carry the gravity and lateral loads. Another main consideration for structural engineers during the design process was the possibility of terrorist attacks and other threats. Like all government buildings that have been designed since the September 11th attacks, progressive collapse design and blast resistant requirements were required for the Army National Guard Readiness Center Addition. For the purpose of this thesis, the building will be re-evaluated using a steel framing system and will include investigation into the progressive collapse capabilities. Blast and explosion loading will also be considered for the new structural system.

METHODOLOGIES & SOLUTIONS

A steel frame system will initially be designed to resist that lateral and gravity loads. The existing column grid will serve as a guide however; deviations will most likely be inevitable. The floor-to-floor height of 13.5 feet will remain the same. Multiple considerations for both braced frames and moment frames will be investigated to determine the most practical solution.

The progressive collapse analysis will include the hypothetical loss of a primary structural element. Critical perimeter and interior members will be 'lost' and a structural analysis will be performed without the critical component. Since the structural elements was 'lost' due to a blast or destructive means, specific loading criteria will be utilized for the structural analysis. The results of the analysis will then be compared to the ultimate strength of the structural system to determine its efficiency if there were to be an attack or damage to the structural system.

An analysis into the foundation system will also be required since the proposed structural steel system will be lighter than the existing concrete structure. Overturning and uplift will be investigated to determine if the current mat slab design is still viable or if it must be redesigned for the proposed steel framing system. In the third technical report, a brief inspection into the foundation system confirmed that overturning and uplift did not affect the current system due to the building weight and soil friction.

ASCE 7-05 will be utilized to determine the lateral and gravity load design as well as governing the drift and displacement requirements. "The GSA – Progressive Collapse Guidelines" will be exploited for the progressive collapse and blast analysis. A computer model will be generated using ETABS and/or RAM to assist in the structural analysis. Since the properties of steel are a much different than the existing concrete structure, an examination of the vibration and acoustics at the 5T Level, below the mechanical penthouse, will also be completed if time permits.

BREADTH TOPIC 1 – Architecture

Structural alterations such as the complete change from a cast-in-place concrete system to a steel framing system will inevitably result in layout and architectural changes throughout the building. Changes to the column grid and different size columns will most likely impede on the existing floor layouts. These changes will need to be considered so that a typical floor plan can be redesigned where necessary.

One important aspect is to retain the existing architectural program. This will require that any changes to the floor plans will still maintain the current spatial relationship. Bubble diagrams and other spatial relationship exercises will be used to explore alternatives and reconfigure the floor plans. Once a typical floor plan has been finalized, detailed sketches will be made using AutoCAD and will be included in the final report.

While the floor plans will inevitably be affected, they are not the only architectural aspect that will need to be considered after the structural changes are made. Currently, the façade consists of precast and metal panels with ribbon windows. The anticipated change in column layout and column size will likely cause changes to the façade. Also, the overall floor thickness will change, further changing the height of the façade and the “striping”. Possible new façade options will be investigated through the use of sketches taking into consideration the new steel framing.

BREADTH TOPIC 2 – Construction Management

This breadth study will focus on the scheduling impact and cost-related issues that will be affected by the proposed structural changes. This will include the completion of a new schedule to account for the steel construction. Steel fabricators in the area will be researched and incorporated in the schedule including lead-time and fabrication.

Current cost and schedule information will be obtained from the construction management team. A mock schedule will be created using this information. RS means will also be used to check any information that could not be obtained from the construction management. Advantages and disadvantages for both the proposed steel system and the existing concrete system will be compared and analyzed. A final conclusion will determine if one system is a considerably better option than the other design.

TASKS & TOOLS

Task 1: Research Blast Loads and Effects

- Investigate properties of explosions and blasts
- Research blast scenarios to determine worst-case effects
- Research possible solutions for steel framing
- Consider most sensitive areas and determine areas most susceptible to attack

Task 2: Design Gravity Framing System

- Determine Live and Dead Loading
- Consider possible framing systems
- Determine preliminary member sizes

Task 3: Design Lateral Framing System

- Determine lateral loads
- Consider possible framing systems
- Determine preliminary member sizes

Task 4: Generate Computer Model

- Check strength requirements
- Check serviceability requirements

Task 5: Design Progressive Collapse and Blast Resistant Components

- Determine blast load cases
- Determine progressive collapse load cases
- Employ blast and progressive load cases
- Check strength requirements
- Perform size upgrades as necessary

Task 6: Breadth 1

- Consider location of existing shear walls compared to new lateral system
- Determine areas significantly affected by proposed structural system
- Perform spatial relationship exercise to reconfigure problem areas
- Investigate changes and possible solutions to the facade
- Finalize floor plans and façade design

Task 7: Breadth 2

- Research preliminary redesign affects using RS Means
- Consult with the general contractor and obtain available information
- Determine the in-depth schedule changes created by the proposed system
- Consult with vendors and suppliers to determine procurement and erection timing
- Determine the cost implications of the proposed system

Task 8: Conclude Analysis Result

- Finalize all member sizes
- Compare computer model analysis

Task 9: Write Final Report

Task 10: Develop Final Presentation

SCHEDULE

| Weekly Schedule: January 2010 - February 2010 | | | | | | | |
|---|------------|------------|------------|------------|------------|-------------|-------------|
| | Week 1 | Week 2 | Week 3 | Week 4 | Week 5 | Week 6 | Week 7 |
| | January 11 | January 18 | January 25 | February 1 | February 8 | February 15 | February 22 |
| Task 1 | | | | | | | |
| Task 2 | | | | | | | |
| Task 3 | | | | | | | |
| Task 4 | | | | | | | |
| Task 5 | | | | | | | |
| Task 6 | | | | | | | |
| Task 7 | | | | | | | |
| Task 8 | | | | | | | |
| Task 9 | | | | | | | |
| Task 10 | | | | | | | |

| Weekly Schedule: March 2010 - April 2010 | | | | | | |
|--|---------|--------------|----------|----------|----------|---------------|
| | Week 8 | Week 9 | Week 10 | Week 11 | Week 12 | Week 13 |
| | March 1 | March 8 | March 15 | March 22 | March 29 | April 5 |
| Task 1 | | Spring Break | | | | Presentations |
| Task 2 | | | | | | |
| Task 3 | | | | | | |
| Task 4 | | | | | | |
| Task 5 | | | | | | |
| Task 6 | | | | | | |
| Task 7 | | | | | | |
| Task 8 | | | | | | |
| Task 9 | | | | | | |
| Task 10 | | | | | | |

** Note: If time permits a study of the vibration and acoustics below the mechanical penthouse will also be completed*

Conclusion

This proposed thesis will focus on the analysis of the structural system of the Army National Guard Readiness Center Addition as a steel system and a comparison to the existing concrete structure. Progressive collapse and blast analysis will be highlighted in the redesign of the structural framing system. The foundation system will also be analyzed. The reduction in the weight of the building could allow for a decreased mat foundation. However, the overturning moment and uplift could become a more significant issue with the weight reduction and therefore increase the size of the foundation. Time permitting the steel framing system will also be analyzed for vibration and noise transmission at the mechanical penthouse level. Applicable codes will be referenced and utilized for all designs and analyses.

The breadths of this study will include construction management and architectural studies. An in depth cost and schedule analysis will be completed for the construction management study. The results from the steel structure will be compared to that of the existing concrete to determine which system is more practical from a construction standpoint. The architectural study will take into account any changes to the existing column layout and structural sizes to develop new spatial relationships to fit the proposed structural system. The façade will also be analyzed for issues that may arise from the change in structural system and a new façade will be developed to fit the proposed system.