

October 27th, 2010

TECHNICAL ASSIGNMENT TWO

PENN STATE SENIOR THESES



UNIVERSITY SCIENCES BUILDING

NORTHEASTERN U.S.

PENNSTATE



Turner

JUSTIN GREEN

**CONSTRUCTION MANAGEMENT
ADVISOR: DR. RILEY**



University Sciences Building

Northeastern U.S.

TECHNICAL ASSIGNMENT TWO

Justin Green - CM

EXECUTIVE SUMMARY

Technical Assignment Two is intended to analyze the key features of the project that affect the project execution of the University Sciences Building. This project is a mixed use facility that will house both students and faculty of the university. It includes 39 research and teaching laboratories for bio-medical engineering, biology, chemistry, and fossil preparation. This 138,000 square foot building also houses 8 educational classrooms, a small auditorium seating 240 students, and a wing dedicated to both administrative and faculty support.

This report will contain a **detailed project schedule** that is more developed than the summary schedule developed in the first technical assignment. This schedule is comprised of two hundred line items that summarize the key construction tasks and their relationships on the project. Some of the larger challenges with construction will be discussed under the **site layout** portion of this technical report. Under this section, an in-depth analysis of site logistical planning during the building's superstructure phase will describe some of the reasoning behind where items were placed on the jobsite, and how they could possibly be re-arranged to allow for better efficiency in layout around the building perimeter.

Two estimates can be found in the later portion of the report. The first is a **detail structural estimate** that breaks down the costs of steel and concrete within the building. This estimate provided some difficulties because of the building's unique layout and the variety of structural systems implemented into the design. This building has a mix of precast filigree slabs, cast-in-place concrete, and structural steel. The second estimate is a **general conditions estimate**. This estimate breaks down the general conditions costs into four major categories: Supervision & Personnel, Construction Facilities & Equipment, Temporary Utilities, and other Miscellaneous Costs. The largest of which is the cost of supervision & personnel, and the most difficult to accurately predict was the temporary utilities costs during construction. All unit rates were obtained from RS Means Building Construction and all quantities were scaled from the supplied contract drawings.

Lastly, this report contains a summary of **critical industry issues** discussed during the PACE Roundtable Meeting. Items discussed include surprising facts learned, issues that might affect or be applied to this project, and key contacts obtained that can be used for future reference.

After analyzing the information within the report, site logistics is a critical feature and needs to be thoroughly thought out for any project with a limited amount of space for materials and equipment. Site logistics in a congested city has a direct impact on the phasing of exterior enclosures and other trades on site. These considerations can be shown on the detailed project schedule attached.



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TABLE OF CONTENTS

Executive Summary	2
Table of Contents	3
Detailed Project Schedule	4
Site Layout Planning	7
Detailed Structural Systems Estimate	8
General Conditions Estimate	10
Critical Industry Issues	11
Appendix A - Detailed Project Schedule	12
Appendix B - 11x17 Site Logistics Plan	19
Appendix C - Detailed Structural Takeoff with RS Means Data	21
Appendix D - General Conditions Estimate Breakdown	42



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DETAILED PROJECT SCHEDULE

*See APPENDIX A for the Detailed Project Schedule

PRECONSTRUCTION / PROCUREMENT

The University Sciences Building was bid out in the traditional Design-Bid-Build fashion. This means that there was very little to no overlap between the design and construction phases of the project. It is because of this that Turner Construction is unaware of the specific details in the procurement and design of this facility. In comparison with other facilities of the same occupancy type and location, it was determined that the design phase of the project was in the vicinity of 1 year, or 260 work days.

Members of the construction team are also unaware of the exact time frame of when the design was put out to bid and when the team was awarded the project. Most construction projects have a typical contractor selection period of about a month and a half, resulting in the duration of 35 work days for the bid and award of the CM @ Risk.

Lastly, a major time commitment of any project startup is the permits and approvals needed to begin construction. When researching typical permit request and approval durations in the area, it was found that construction permits can take anywhere from 15 to 90 days depending on what type of permit is being requested. This is what you see in the final line item of the preconstruction phase (*Reference Appendix A*).

CONSTRUCTION

For this technical report, the preconstruction and project completion dates did not change from the initial overview schedule generated in the first technical report. For this report, construction was broken up into five major subdivisions. These divisions are general, foundation/superstructure, enclosure/façade, MEP systems, and finishes. Within these divisions are minor subdivisions that break the building up into its different areas.

The major sequence of work on this project is such that construction starts on the first floor and raises one floor at a time in a clockwise rotation until the penthouse is reached (*see Figure 1*). This however, is not the sequence pattern for every type of activity on the project.

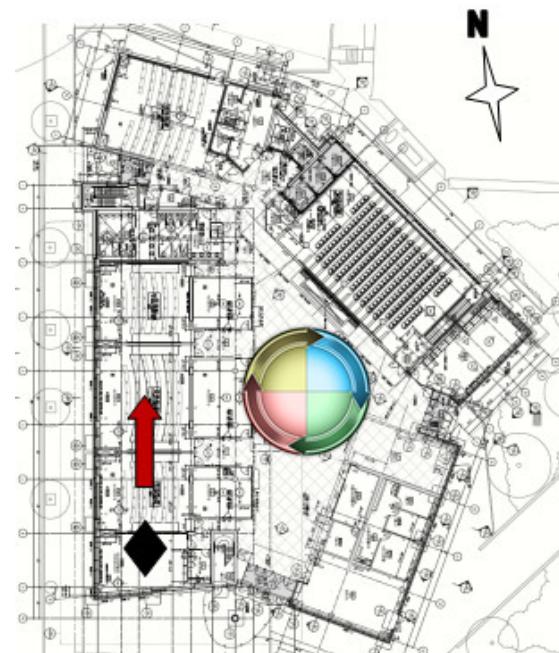


Figure 1: First Floor Layout Showing Sequence



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CONSTRUCTION ACTIVITY BREAKDOWN

Foundation / Superstructure

The structure of the building is comprised of drilled caisson piers, pile caps, grade beams, shear walls, concrete/steel columns, and precast/cast-in-place concrete slabs. The masonry walls are under the enclosure/façade portion of the schedule because there are no load bearing walls in the University Sciences Building, making the CMU walls an architectural feature of the building instead of a means of support for the facility.

This grouping of activities is broken down by floor, starting at the basement level and proceeding to the roof of the penthouse floor.

One important thing to point out is that the slab on grade (SOG) was placed after the other building slabs because of the shoring required to support each floor above. You need four levels of shoring before you can start the SOG. Another reason that SOG's are typically skipped over and poured later on in the job is because of the time needed to wait for the ground floor to cure. This can hold up other activities within the building.

Enclosure / Façade

The enclosure is unique in that the sequencing of masonry wall proceeds from floor to floor, while the rest of the façade focuses on one face of the building at a time. More time was allocated to the masonry work on the first floor when compare to the other floors that have a similar quantity of CMU block. This may be because of the learning curve with any new activity, and possibly the assembly of scaffolding around the facility.

Another surprising fact about the sequencing of the façade is that it typically proceeds in a clockwise direction like the other building activities, but often skips the south-eastern façade of the building (the side facing the laydown area) and returns after the other faces have been completed. Perhaps this is due to limited space on that side of the University Sciences Building at the time of erection.

MEP Systems

The MEP systems consist of electrical, mechanical, plumbing, fire protection, and elevator work. Of these systems, the electrical, mechanical, and plumbing features of the building have been broken down into their core activities. These activities include rough-in, distribution, and final connections. The hardest of which to analyze was that of the mechanical systems within the building. This is because of the large air and ventilation requirements that are involved with the laboratory spaces, as well as the quantity of activities present in the current schedule.



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Building Finishes

The building finishes were broken down into framing of interior partitions, hanging/taping/painting drywall, installation of ceiling grid, hanging of lighting fixtures, dropping ceiling tile, and flooring installation. Other key finishes to point out lie within the auditorium and atrium spaces within the University Sciences Building.

The atrium is unique because of its living wall system and the large quantity of scaffolding that will fill every square foot of the open space. The four floor atrium spaced will be completely filled with scaffolding in order to streamline the finishing of walls and railings around the perimeter of the space. The less you move scaffolding or other forms of equipment, more time can be spent actually performing the work and performing the work safely.

Summary of Key Milestones

MILESTONES	KEY CONSTRUCTION DATES
Notice to Proceed	8/25/2009
Ground Breaking	10/21/2009
Steel Top Out	7/20/2010
Enclosure	1/27/2011
Substantial Completion	6/30/2011

Figure 2: Summary of Key Project Milestones



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SITE LAYOUT PLANNING

***See APPENDIX B for a Site Plan of the Superstructure Logistics Plan**

The site for the University Sciences Building is located in the heart of the University's campus. This means that vehicular and pedestrian traffic will be a major concern during all phases of the project. Whether it means blocking of street lanes or installing overhead protection, site safety has to be taken into serious consideration.

Being in a city has its advantages and disadvantages. One perk of this building's location is the easy access to nearby utility lines and highly skilled trades in the area. Some of the major disadvantages include limited space for site storage and material deliveries. This means that more thought has to be put into scheduling and material staging. Less space outside of the building also typically equates to more congestion within the building.

The Superstructure Phase Plan

The site logistics plan shown below (*and in larger detail in Appendix B*) of the superstructure phase of the project is very similar to all other phases of the project. Site access and laydown space along with trailer locations are identical throughout the entire duration of the project. The key feature shown in *figure 3* is that of the positioning of the tower crane. This type of crane was used because of the limited space around the perimeter of the University Sciences Building, as well as the convenience of having a large open space within the building to help avoid crane interference with building trades.

Contractor Layout Critique

This site is already extremely efficient, mostly because of the simplicity of its layout and the limited space to stage any materials or equipment outside of the building.

The positioning of the tower crane in the atrium makes sense because of its central location, the ease of access to all areas of the structure, and the limited disturbance to the work surrounding the tower (no holes left to patch after the floors are erected and crane is removed).

Two site entrances with two areas for material distribution help give the site flexibility and ease of access at all times.

The only space that could be rearranged or condensed is that of the northwestern corner of the site. It may have been better to arrange the concrete subcontractor's trailer next to the other trailers on site. Also, the mock-up panel has room to move either further backward or closer to the Turner job trailers. Moving these few items would allow for more space that could be used to store materials or equipment.

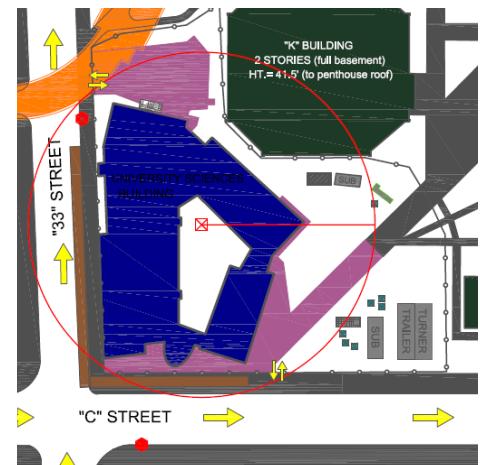


Figure 3: Superstructure Layout Plan



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DETAILED STRUCTURAL SYSTEM ESTIMATE

*See APPENDIX C for the complete Structural System Estimate

The superstructure for the University Sciences Building is composed of a variety of different steel and concrete systems. Steel for the building contains a variety of I-beams sizes, mainly located at the Penthouse levels, with some steel existing above the first floor auditorium. The University Sciences Building's concrete floor system is composed of a thin precast layer (filigree slab) that supports a voided cast-in-place concrete slab on top.

This detailed structural estimate (*located in Appendix C*) is a good representation of the actual system costs for concrete and steel within the building.

Some of the major factors that can affect cost are as follows:

- Quality
- Productivity
- Size of Project
- Location
- Overtime

The factor that most affects this estimate is productivity because of the use of precast panels on the project. These filigree slabs can drastically reduce the time of placing concrete and completing the structure. Filigree slabs replace the need for formwork, thereby reducing the time between concrete placements. This is why the estimated concrete values are a little higher than the actual cost of the system (*see figure 4 below*), because of the labor savings not accounted for by the RS Means values utilized in the estimate.

Steel costs were found to be almost exactly the same as the actual cost of construction. The accuracy of this estimate can be contributed to the detailed breakdown of each and every structural steel member within the building (with the exception of tube steel supporting the atrium skylights).

MAJOR STRUCTURAL SYSTEMS				
SYSTEM	ACTUAL COST	COST PER SF	ESTIMATED COST	COST PER SF
Concrete	\$ 5,800,000	\$ 42.03	\$ 6,927,426	\$ 50.20
Steel	\$ 800,000	\$ 5.80	\$ 797,602	\$ 5.78

Figure 4: Structural Comparison

Actual vs. Estimated



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These pie charts shown in *figure 5* represent the breakdown of costs for each type of structural system. Actual dollar amounts can be found in *Appendix C*.

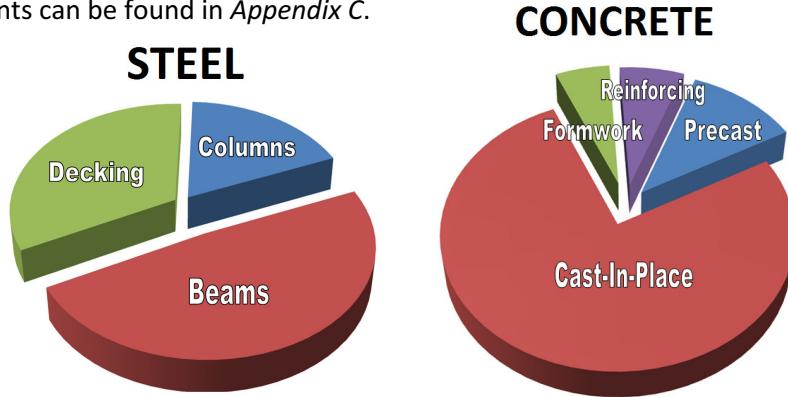


Figure 5: Cost Percentage Breakdowns for Steel & Concrete Systems

Items that the detailed estimate does not include are:

Concrete finishing/curing, bulk heads, expansion/saw cut joints, block outs, masonry CMU, metal wall studs, miscellaneous steel, or percentages for material waste.

All of the unit pricing in this detailed estimate came from RS Means Building Construction 2011. These values were used to determine all material, labor, and equipment costs for the structural system of the University Sciences Building. A location factor was applied at the end of the estimate to accurately adjust the average costs that RS Means supplies to better fit the city that this building is currently being constructed.

Other assumptions and findings can be found at the end of the estimate, located in *Appendix C*. Floor areas were determined using the scaled CAD drawings shown below in *figure 6*.

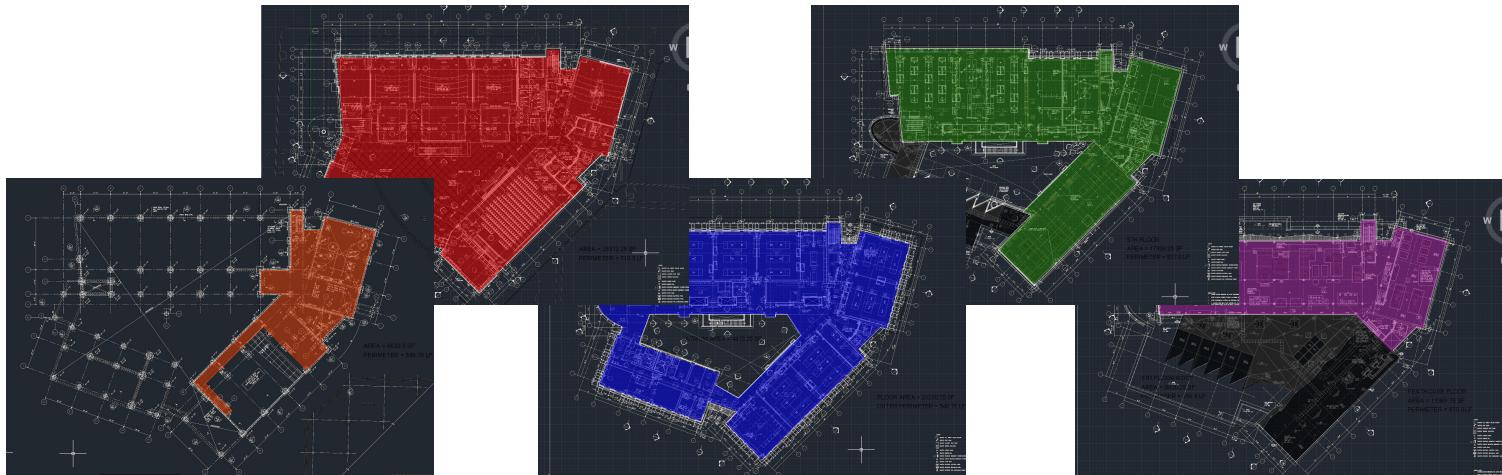


Figure 6: Floor Areas Found in CAD for Quantity Take-offs



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GENERAL CONDITIONS ESTIMATE

*See APPENDIX D for the complete Structural System Estimate

The estimate, summarized in *figures 7 and 8*, shows the cost breakdown for the general conditions of the University Sciences Building project. All values are an approximation based upon RS Means figures, and do not reflect the actual amounts incurred by Turner construction.

The estimate was broken down into four separate categories: Supervision and Personnel, Construction Facilities and Equipment, Temporary Utilities, and other Miscellaneous Costs. All figures are based on a project duration of 22 months, and a bulk construction period of 20 months.

Supervision and Personnel includes all staffing and project administration for the project. This cost was found to be the largest of the four categories considered, and will be the case for most projects constructed in the United States.

Construction Facilities and Equipment incorporates items such as field office trailer set-up & removal, construction fencing & pedestrian protection, storage, tools & equipment, personal protective equipment, etc.

Temporary Utilities are the most difficult of the four categories to accurately predict without knowing the exact extent and durations for the use of things such as temporary power, water, phone, toilets, etc. on the project.

Miscellaneous Costs account for items such as commissioning, document reproduction, permits, bonding, insurance, travel expenses, etc. These costs account for the second largest contribution to the general conditions estimate. This is true in large part because of the cost of commissioning, permits, bonding, and insurance on the job. Each of these categories is determined by RS Means as a percentage of the overall cost of the project. So the larger the project, the higher the miscellaneous costs incurred.

GENERAL CONDITIONS SUMMARY

Description	Cost/Month	Cost
Supervision and Personnel	\$ 68,611	\$ 1,509,450
Construction Facilities and Equipment	\$ 7,325	\$ 161,150
Temporary Utilities	\$ 2,350	\$ 51,700
Miscellaneous Costs	\$ 56,578	\$ 1,244,720
TOTAL		\$ 2,967,020

Figure 7: General Conditions Breakdown by Category.

General Conditions Summary



Figure 8: General Conditions by percentage.



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CRITICAL INDUSTRY ISSUES – PACE ROUNDTABLE SUMMARY

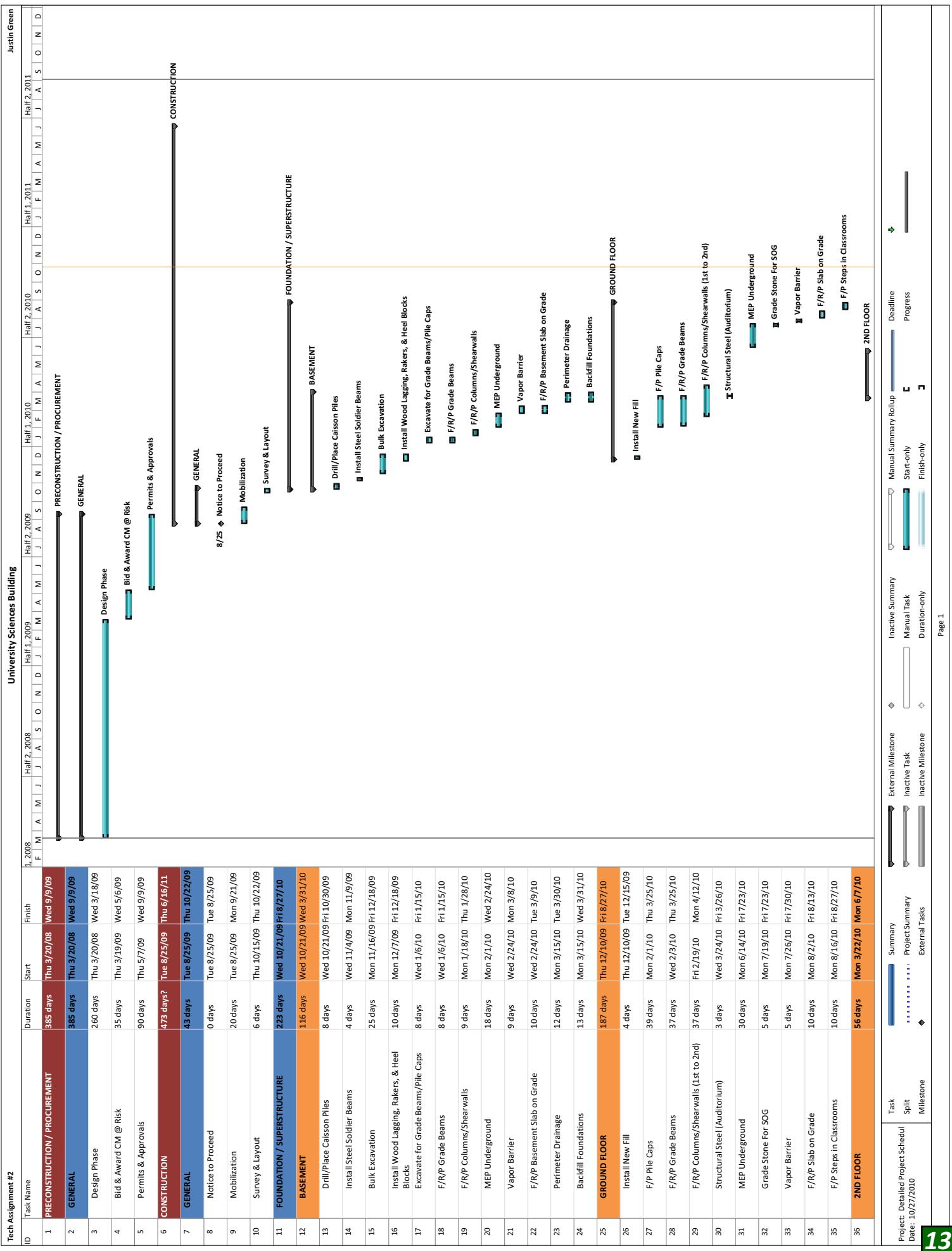
Summary of events to follow shortly...



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APPENDIX A - Detailed Project Schedule



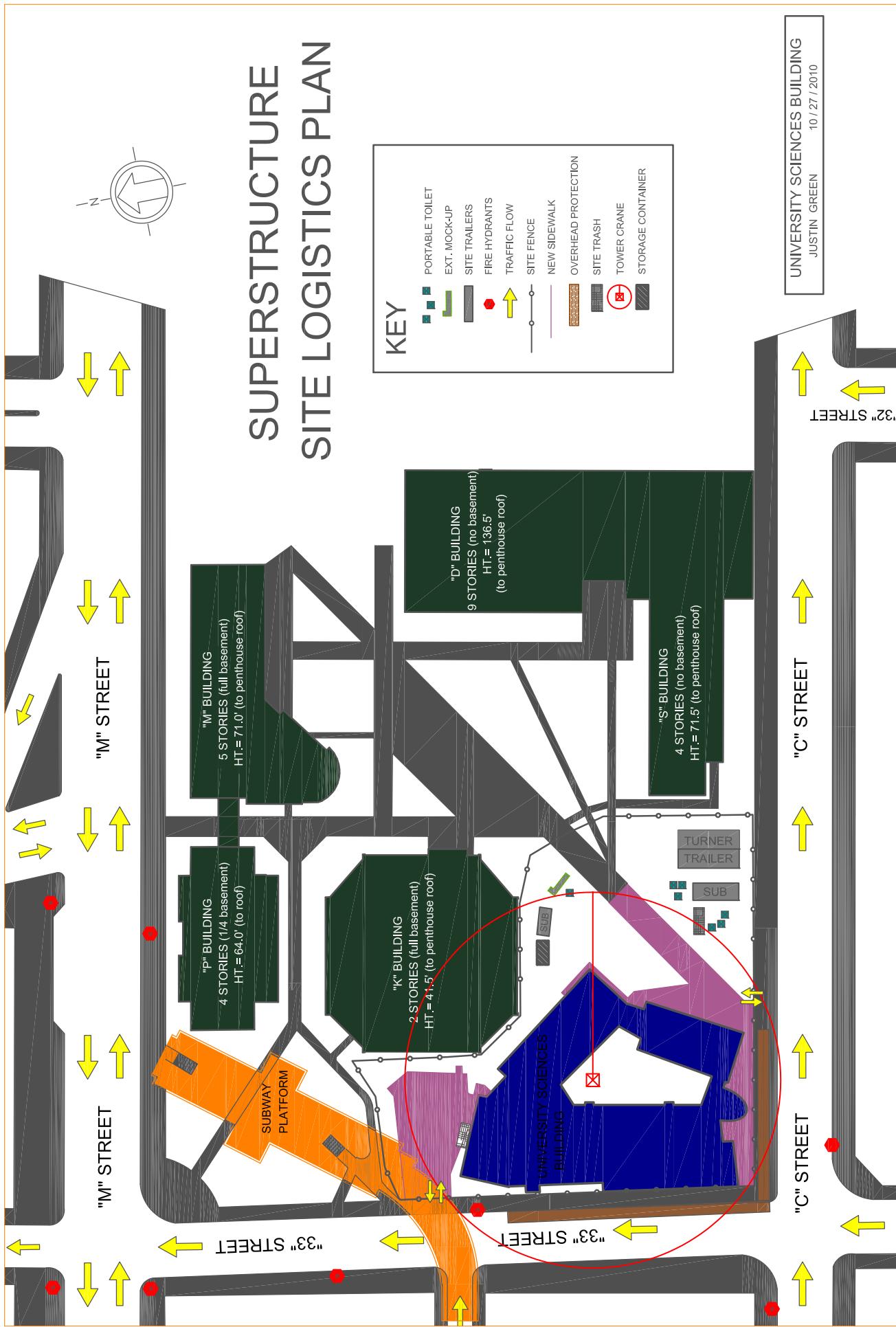
University Sciences Building																
Tech Assignment #2		Duration			Start			Finish			Half 1, 2008			Half 2, 2008		
ID	Task Name				F	M	J	A	S	O	N	D	J	F	M	
145	ELEVATORS	98 days	Wed 10/20/10	Fri 3/4/11												
146	Installation and Testing	98 days	Wed 10/20/10	Fri 3/4/11												
147	FINISHES	226 days?	Thu 8/5/10	Thu 6/16/11												
148	GROUND FLOOR															
149	Frame Interior Partitions	10 days	Wed 10/27/10	Tue 11/9/10												
150	Drywall - Hang, Tape, Paint (1st Coat)	20 days	Wed 11/24/10	Tue 12/21/10												
151	Install Ceiling Grid	10 days	Wed 12/22/10	Tue 1/4/11												
152	Lighting Fixtures	10 days	Wed 1/19/11	Tue 2/1/11												
153	Drop Ceiling Tile	20 days	Wed 2/2/11	Tue 3/1/11												
154	Painting - Final Coat	5 days	Wed 3/2/11	Tue 3/8/11												
155	Classroom Seating	10 days	Wed 4/6/11	Tue 4/19/11												
156	Flooring Installation	10 days	Wed 4/20/11	Tue 5/3/11												
157	2ND FLOOR															
158	Frame Interior Partitions	45 days	Thu 8/5/10	Wed 10/6/10												
159	Drywall - Hang, Tape, Paint (1st Coat)	52 days	Wed 10/20/10	Thu 12/30/10												
160	Install Ceiling Grid	84 days	Mon 11/8/10	Thu 3/3/11												
161	Lighting Fixtures	84 days	Mon 11/15/10	Thu 3/10/11												
162	Drop Ceiling Tile	114 days	Mon 11/29/10	Thu 5/5/11												
163	Flooring Installation	91 days	Thu 12/2/10	Thu 4/7/11												
164	Painting - Final Coat	106 days	Mon 12/20/10	Mon 5/16/11												
165	3RD FLOOR															
166	Frame Interior Partitions	65 days	Thu 8/5/10	Wed 11/3/10												
167	Drywall - Hang, Tape, Paint (1st Coat)	77 days	Thu 9/16/10	Fri 12/31/10												
168	Install Ceiling Grid	109 days	Tue 10/5/10	Fri 3/4/11												
169	Lighting Fixtures	109 days	Tue 10/12/10	Fri 3/11/11												
170	Drop Ceiling Tile	144 days	Tue 10/19/10	Fri 5/6/11												
171	Flooring Installation	84 days	Tue 12/14/10	Fri 4/8/11												
172	Painting - Final Coat	113 days	Fri 12/10/10	Tue 5/17/11												
173	4TH FLOOR															
174	Frame Interior Partitions	75 days	Thu 8/5/10	Wed 11/17/10												
175	Drywall - Hang, Tape, Paint (1st Coat)	61 days	Wed 10/20/10	Wed 1/12/11												
176	Install Ceiling Grid	93 days	Mon 11/8/10	Wed 3/16/11												
177	Lighting Fixtures	93 days	Mon 11/15/10	Wed 3/23/11												
178	Drop Ceiling Tile	128 days	Mon 11/22/10	Wed 5/18/11												
179	Flooring Installation	75 days	Thu 1/6/11	Wed 4/20/11												
180	Painting - Final Coat	125 days	Mon 12/6/10	Fri 5/27/11												
Project: Detailed Project Schedule		Task Split	Summary	Inactive Task	Inactive Milestone	Inactive Milestone	Inactive Task	External Task	External Milestone	Manual Summary	Manual Task	Deadline	Progress			
Date: 10/27/2010		Milestone		
				



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APPENDIX B - Site Logistics Plan of Superstructure





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APPENDIX C - Detailed Structural Takeoff with RS Means Data

DETAILED STRUCTURAL SYSTEMS ESTIMATE SUMMARY

Type	Description	Quantity	Unit	Material Unit Rate	Material Cost	Labor Unit Rate	Labor Cost	Equipment Unit Rate	Equipment Cost	Total Cost
Structural Steel										
Steel Column (28)										
Steel Column (28)	W8x40 - 22'6"	630.000	Lf	\$ 59.50	\$ 37,485.00	4.82	\$ 3,036.60	2.95	\$ 1,858.50	\$ 42,380.10
Steel Column (17)	W8x48 - 22'6"	382.500	Lf	\$ 59.50	\$ 22,758.75	4.82	\$ 1,843.65	2.95	\$ 1,283.88	\$ 25,730.78
Steel Column (7)	W8x48 - 29'9"	208.250	Lf	\$ 59.50	\$ 12,390.88	4.82	\$ 1,003.77	2.95	\$ 614.34	\$ 14,008.98
Steel Column (1)	W8x67 - 29'9"	29.750	Lf	\$ 59.50	\$ 1,770.13	4.82	\$ 143.40	2.95	\$ 87.76	\$ 2,001.28
Steel Column (1)	W12x92 - 22'6"	22.500	Lf	\$ 59.50	\$ 1,338.75	4.82	\$ 108.45	2.95	\$ 66.38	\$ 1,513.58
Steel Beam (8)										
Steel Beam (8)	Auditorium - W10x26 - 15'0"	120.000	Lf	\$ 32.00	\$ 3,840.00	4.42	\$ 530.40	2.70	\$ 324.00	\$ 4,694.40
Steel Beam (8)	Auditorium - W12x26 - 40'0"	320.000	Lf	\$ 32.00	\$ 10,240.00	3.01	\$ 963.20	1.84	\$ 588.80	\$ 11,792.00
Steel Beam (2)	Auditorium - W24x103 - 44'0"	88.000	Lf	\$ 129.00	\$ 11,352.00	3.65	\$ 321.20	1.65	\$ 145.20	\$ 11,818.40
Steel Beam (7)	5th Fl - W18x35 - 21'0"	147.000	Lf	\$ 43.50	\$ 6,394.50	3.99	\$ 586.53	1.80	\$ 264.60	\$ 7,245.63
Steel Beam (3)	5th FL - W18x40 - 21'0"	63.000	Lf	\$ 49.50	\$ 3,118.50	3.99	\$ 251.37	1.80	\$ 113.40	\$ 3,483.27
Steel Beam (7)	5th FL - W21x44 - 36'6"	255.500	Lf	\$ 54.50	\$ 13,924.75	3.60	\$ 919.80	1.63	\$ 416.47	\$ 15,261.02
Steel Beam (1)	5th FL - W21x48 - 21'0"	21.000	Lf	\$ 62.00	\$ 1,302.00	3.60	\$ 75.60	1.63	\$ 34.23	\$ 1,411.83
Steel Beam (1)	5th FL - W21x50 - 36'6"	36.500	Lf	\$ 62.00	\$ 2,263.00	3.60	\$ 131.40	1.63	\$ 59.50	\$ 2,453.90
Steel Beam (1)	5th FL - W21x62 - 36'6"	36.500	Lf	\$ 76.50	\$ 2,792.25	3.70	\$ 135.05	1.67	\$ 60.96	\$ 2,988.26
Steel Beam (1)	5th FL - W21x68 - 36'6"	84.000	Lf	\$ 3,066.00	\$ 3,705	4.42	\$ 1,423.24	2.70	\$ 869.40	\$ 6,285.44
Steel Beam	Atrium - W8x10 - 32'2"	322	Lf	\$ 12.40	\$ 3,992.80	4.42	\$ 1,423.24	2.70	\$ 869.40	\$ 6,285.44
Steel Beam	Atrium - W8x15 - 9'	9	Lf	\$ 18.55	\$ 166.95	4.42	\$ 39.78	2.70	\$ 24.30	\$ 231.93
Steel Beam	Atrium - W8x15 - 59'	59	Lf	\$ 43.50	\$ 2,566.50	4.82	\$ 284.38	2.95	\$ 174.05	\$ 3,024.93
Steel Beam	Atrium - W10x12 - 21'	12	Lf	\$ 14.85	\$ 178.20	4.42	\$ 53.04	2.70	\$ 32.40	\$ 263.64
Steel Beam	Atrium - W12x14 - 47'	47	Lf	\$ 19.80	\$ 930.60	3.01	\$ 141.47	1.84	\$ 86.48	\$ 1,158.55
Steel Beam	Atrium - W12x152 - 11'	11	Lf	\$ 108.00	\$ 1,188.00	4.14	\$ 45.54	2.53	\$ 27.83	\$ 1,261.37
Steel Beam	Atrium - W14x22 - 11'7"	117	Lf	\$ 32.00	\$ 3,744.00	2.68	\$ 313.56	1.64	\$ 191.88	\$ 4,249.44
Steel Beam	Atrium - W18x35 - 47'	47	Lf	\$ 43.50	\$ 2,044.50	3.99	\$ 187.53	1.80	\$ 84.60	\$ 2,316.63
Steel Beam	Atrium - W18x40 - 31'	31	Lf	\$ 49.50	\$ 1,534.50	3.99	\$ 123.69	1.80	\$ 55.80	\$ 1,713.99
Steel Beam	Atrium - W18x65 - 40'	40	Lf	\$ 80.50	\$ 3,220.00	4.26	\$ 170.40	1.92	\$ 76.80	\$ 3,467.20
Steel Beam	Atrium - W21x44 - 23'	23	Lf	\$ 54.50	\$ 1,253.50	3.60	\$ 82.80	1.63	\$ 37.49	\$ 1,373.79
Steel Beam	Atrium - W21x48 - 17'	17	Lf	\$ 62.00	\$ 1,054.00	3.60	\$ 61.20	1.63	\$ 27.71	\$ 1,142.91
Steel Beam	Atrium - W21x93 - 43'	43	Lf	\$ 115.00	\$ 4,945.00	3.83	\$ 164.69	1.73	\$ 74.39	\$ 5,184.08
Steel Beam	Atrium - W21x101 - 110'	110	Lf	\$ 125.00	\$ 13,750.00	3.83	\$ 421.30	1.73	\$ 190.30	\$ 14,361.60
Steel Beam	Atrium - W21x111 - 20'	20	Lf	\$ 151.00	\$ 3,020.00	3.83	\$ 76.60	1.73	\$ 34.60	\$ 3,131.20
Steel Beam	Atrium - W21x166 - 150'	150	Lf	\$ 151.00	\$ 22,650.00	3.83	\$ 574.50	1.73	\$ 259.50	\$ 23,484.00
Steel Beam	Pent - W8x10 - 253'	253	Lf	\$ 12.40	\$ 3,137.20	4.42	\$ 1,118.26	2.70	\$ 683.10	\$ 4,938.56
Steel Beam	Pent - W8x15 - 84'	84	Lf	\$ 18.55	\$ 1,558.20	4.42	\$ 371.28	2.70	\$ 226.80	\$ 2,156.28
Steel Beam	Pent - W8x24 - 165'	165	Lf	\$ 29.50	\$ 4,867.50	4.82	\$ 795.30	2.95	\$ 486.75	\$ 6,149.55
Steel Beam	Pent - W10x12 - 12'	12	Lf	\$ 14.85	\$ 178.20	4.42	\$ 53.04	2.70	\$ 32.40	\$ 263.64
Steel Beam	Pent - W10x15 - 96'	96	Lf	\$ 18.55	\$ 1,780.80	4.42	\$ 424.32	2.70	\$ 259.20	\$ 2,464.32
Steel Beam	Pent - W12x14 - 68'	68	Lf	\$ 19.80	\$ 1,346.40	3.01	\$ 204.68	1.84	\$ 125.12	\$ 1,676.20
Steel Beam	Pent - W12x16 - 24'	24	Lf	\$ 19.80	\$ 475.20	3.01	\$ 72.24	1.84	\$ 44.16	\$ 591.60
Steel Beam	Pent - W14x22 - 252'	252	Lf	\$ 32.00	\$ 8,064.00	2.68	\$ 675.36	1.64	\$ 413.28	\$ 9,152.64
Steel Beam	Pent - W14x30 - 38'	38	Lf	\$ 37.00	\$ 1,406.00	2.95	\$ 112.10	1.80	\$ 68.40	\$ 1,586.50
Steel Beam	Pent - W14x38 - 14'	14	Lf	\$ 42.00	\$ 588.00	3.27	\$ 45.78	2.00	\$ 28.00	\$ 661.78
Steel Beam	Pent - W16x26 - 115'	115	Lf	\$ 32.00	\$ 3,680.00	2.65	\$ 304.75	1.62	\$ 186.30	\$ 4,171.05
Steel Beam	Pent - W16x31 - 293'	293	Lf	\$ 38.50	\$ 11,280.50	2.95	\$ 864.35	1.80	\$ 527.40	\$ 12,672.25
Steel Beam	Pent - W18x35 - 226'	226	Lf	\$ 43.50	\$ 9,831.00	3.99	\$ 901.74	1.80	\$ 406.80	\$ 11,139.54
Steel Beam	Pent - W18x40 - 195'	195	Lf	\$ 49.50	\$ 9,652.50	3.99	\$ 778.05	1.80	\$ 351.00	\$ 10,781.55
Steel Beam	Pent - W18x46 - 148'	148	Lf	\$ 57.00	\$ 8,436.00	3.99	\$ 590.52	1.80	\$ 266.40	\$ 9,292.92

Steel Beam	Pent - W18x55 - 21'		21	Lf		62.00	\$ 1,302.00	4.20	\$ 88.20	1.90	\$ 39.90	\$ 1,430.10
Steel Beam	Pent - W18x55 - 42'		42	Lf		68.00	\$ 2,856.00	4.20	\$ 176.40	1.90	\$ 79.80	\$ 3,112.20
Steel Beam	Pent - W21x44 - 17'		17	Lf		54.50	\$ 926.50	3.60	\$ 61.20	1.63	\$ 27.71	\$ 1,015.41
Steel Beam	Pent - W24x84 - 19'		19	Lf		103.00	\$ 1,957.00	3.83	\$ 72.77	1.73	\$ 32.87	\$ 2,062.64
Steel Beam	Pent Roof - W10x30 - 120'		120	Lf		41.00	\$ 4,920.00	4.82	\$ 578.40	2.95	\$ 354.00	\$ 5,832.40
Steel Beam	Pent Roof - W10x33 - 54'		54	Lf		41.00	\$ 2,214.00	4.82	\$ 260.28	2.95	\$ 159.30	\$ 2,633.58
* Doesn't include Steel at Skylights												
Totals:							\$ 280,732		\$ 22,898		\$ 12,870	\$ 316,500

Type	Description	Quantity	Unit	Material Unit Rate	Material Cost	Labor Unit Rate	Labor Cost	Equipment Unit Rate	Equipment Cost	Total Cost
Pre-cast										
Pre-cast Filigree Slabs										
2nd Fl - 2.25" typ. Filigree Slab										
3rd Fl - 2.25" typ. Filigree Slab										
4th Fl - 2.25" typ. Filigree Slab										
5th Fl - 2.25" typ. Filigree Slab										
Pent - 2.25" typ. Filigree Slab										
Totals:										
					\$ 674,165		\$ 120,885		\$ 39,055	\$ 834,105

Type	Description	Quantity	Unit	Material Unit Rate	Material Cost	Labor Unit Rate	Labor Cost	Equipment Unit Rate	Equipment Cost	Total Cost
Cast-In-Place Concrete										
Caisson (20)										
36"(3'-0") diameter caisson										
42"(3'-6") diameter caisson										
48"(4'-0") diameter caisson										
54"(4'-6") diameter caisson										
60"(5'-0") diameter caisson										
Pile Cap (13)										
42"x 42"x 40" pile cap										
48"x 48"x 40" pile cap										
54"x 54"x 40" pile cap										
56"x 56"x 40" pile cap										
58"x 58"x 40" pile cap										
60"x 60"x 40" pile cap										
66"x 66"x 40" pile cap										
82"x 42"x 40" pile cap										
Grade Beam										
Grade Beams (~ 1,450 lf)										
Concrete Wall - 11'8" x 10" section										
Foundation Wall (379 lf)										
Column (5)										
Base to Ground - 10"x 24"										
Column (2)										
Base to Ground - 16"x 34"										
Column (6)										
Base to Ground - 16"x 36"										
Column (2)										
Base to Ground - 24"x 24"										
Column (1)										
Base to Ground - 24" diameter										
Column (1)										
Base to Ground - 28" diameter										
Column (4)										
Base to 2nd - 28"x 40"										
Column (15)										
1st to 2nd - 24" diameter										
Column (3)										

STRUCTURAL SYSTEMS GRAND TOTAL

TOTAL MATERIAL BREAKDOWN =
TOTAL LABOR BREAKDOWN =
TOTAL EQUIPMENT BREAKDOWN

**\$5,295,437
\$2,431,008
\$656,962**

TOTAL CONCRETE (w/out Location Mult.) =
TOTAL STEEL (w/out Location Mult.) =

\$6,927,426
\$797,602

\$8,383,407

ASSUMPTIONS / FINDINGS

Estimated π to be 3.14.

CAISONS

Caisons have a minimum length of 13'6", and that's what I used for length.

GRADE BEAMS

Determined average width and depths of grade beams to be 16" x 36" based on the range of values found in the beam schedule.

Determined average stirrup spacing w/in grade beams to be 15" and an average length of 8.667 LF based on the range of values found in the beam schedule.

Determined average horizontal spacing of rebar in grade beams to be 6" totaling 7 bars on each face of the grade beam based on the range of values found in the beam schedule.

FOUNDATION WALLS

Typical vertical (Inside Face) reinforcing is #8@10".

Typical vertical (Outside Face) reinforcing is #5@18".

SHEAR WALLS

Typical horizontal reinforcing is #4 rebar @ 12" spacing (found in the caisson and column schedule).

Typical average vertical reinforcement is #8 rebar (very few cases have #10 rebar, so assumed #8 for all cases).

CONCRETE COLUMNS

Typical horizontal reinforcement is #4 rebar @ 12" spacing (found in the caisson and column schedule).

Typical average vertical reinforcement is #8 rebar (very few cases have #10 rebar, so assumed #8 for all cases).

CONCRETE BEAMS

Typical concrete beams are 96" x 18" (includes slab thickness) on Floors 2-4.

Typical concrete beams are 96" x 26" (includes slab thickness) on the 5th Floor.

Typical concrete beams are 96" x 28" (includes slab thickness) on the Penthouse Floor.

Determined the cross-sectional area of the typical 96" x 18" voided concrete beam with precast to contain 2.25 SF of concrete (this excludes concrete considered in slab quantity).

Determined the cross-sectional area of the typical 96" x 26" voided concrete beam with precast to contain 4.92 SF of concrete (this excludes concrete considered in slab quantity).

Determined the cross-sectional area of the typical 96" x 28" voided concrete beam with precast to contain 5.58 SF of concrete (this excludes concrete considered in slab quantity).

PREFAST SLABS

SF of Filigree Slab is roughly the same as the total SF of slab area for each floor.

Precast slabs include rebar in their material cost.

SLABS

The majority of slabs (excluding SOG's) are 8" and 10" voided slabs with a 12" thickened edge at the perimeter of the building (36" from the edge of slab).

Assumed a 9" voided slab everywhere to simplify calculations and to account for the extra concrete at thickened slab locations.

SOG thickness for the Auditorium, Offices, and Classrooms is 5" thick.

SOG thickness for the Atrium is 6" thick.

Determined the ratio of CY of concrete to SF of slab surface area to be 0.01302 CY/SF for a typical 9" Voided Slab

Basement Area = 5,633.5 SF

1st Floor Area = 25,312.25 SF (Includes Atrium Area)

2nd Floor Area = 21,230.75 SF

3rd Floor Area = 21,230.75 SF

4th Floor Area = 21,230.75 SF

5th Floor Area = 17,926.25 SF

Penthouse Floor Area = 11,369.75 SF

Atrium Floor Area = 4,413.25 SF

CONCRETE FORMWORK

Assumed multiple uses of various types of formwork.

STEEL REINFORCING

6x6 - W2.0 x W2.0 WWF throughout all elevated slabs (totaling 93,000 SF).

6x6 - W4.0 x W4.0 WWF throughout all Slab On Grade concrete (totaling 31,000 SF).

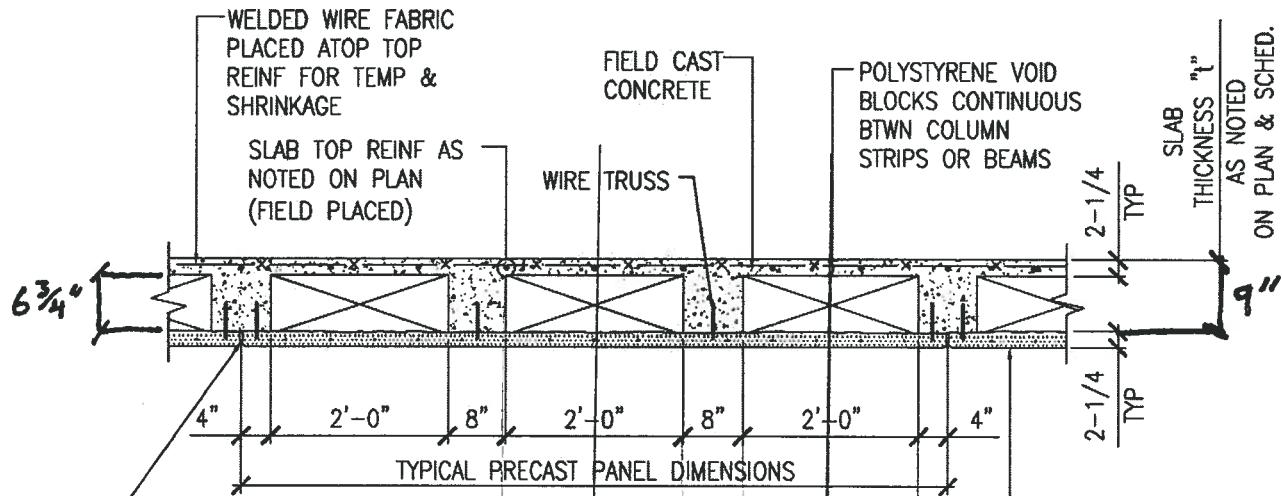
SOG's have a typical reinforcing of #5 rebar spaced @12" o.c. (one way).

Concrete beams have typical longitudinal rebar with 7#9 bar on top and 5#6 bar on the bottom.

Concrete beams have typical horizontal rebar of #5@12" spanning an average of 160".

REINFORCING STEEL CALCULATIONS:

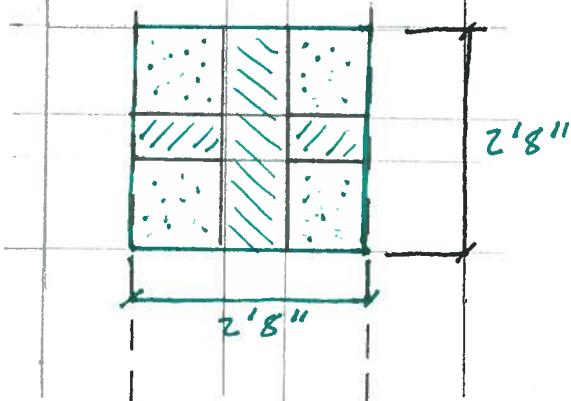
TYPICAL VOIDED SLAB SECTION



PLACE PANELS TIGHT TOGETHER TO ELIMINATE LEAKAGE AT THE JOINTS.

PRECAST "FILIGREE" SOFFIT w/BOTTOM REINFORCING COMPRISED OF GRADE 60 BARS & GRADE 270 STRAND. FACTORED POSITIVE DESIGN MOMENT MU (FT-KIPS/FT) NOTED □ ON PLAN.

$$2'8'' \times 2'8'' = \\ 7.111 \text{ SQ. FT.}$$



$$\therefore \text{Middle} = 2'8'' \times 8'' \times 6\frac{3}{4}'' = 0.037037 \text{ CY}$$

$$\therefore \text{Middle} = 2' \times 8'' \times 6\frac{3}{4}'' = 0.027778 \text{ CY}$$

$$\therefore \text{Top of Voids} = 4 \times 1' \times 1' \times 2\frac{1}{4}'' = 0.027778 \text{ CY}$$

Ratio:

$$\frac{0.092593 \text{ CY}}{7.111 \text{ SF}} = \boxed{0.01302 \text{ CY/SF of Slab}}$$

$$\Sigma = 0.092593 \text{ CY}$$

03 11 Concrete Forming

03 13 – Structural Cast-In-Place Concrete Forming

ProFit = 10%

X 03 11 13.20 Forms In Place, Beams and Girders

			Crew	Daily Output	Labor-Hours	Unit	Material	2011 Bare Costs	Labor	Equipment	Total	Total Ind O&P
3150	4 use		C-2	430	.112	SFCA	.79	4.68			5.47	8.05
3500	Bottoms only, to 30" wide, job-built plywood, 1 use			230	.209		3.42	8.75			12.17	17.20
3550	2 use			265	.181		1.91	7.60			9.51	13.75
3600	3 use			280	.171		1.37	7.20			8.57	12.55
3650	4 use			290	.166		1.11	6.95			8.06	11.85
4000	Sides only, vertical, 36" high, job-built plywood, 1 use			335	.143		3.46	6			9.46	13
4050	2 use			405	.119		1.91	4.97			6.88	9.70
4100	3 use			430	.112		1.39	4.68			6.07	8.70
4150	4 use			445	.108		1.13	4.52			5.65	8.20
4500	Sloped sides, 36" high, 1 use			305	.157		3.41	6.60			10.01	13.85
4550	2 use			370	.130		1.90	5.45			7.35	10.45
4600	3 use			405	.119		1.36	4.97			6.33	9.10
4650	4 use			425	.113		1.11	4.74			5.85	8.45
5000	Upstanding beams, 36" high, 1 use			225	.213		4.14	8.95			13.09	18.25
5050	2 use			255	.188		2.30	7.90			10.20	14.65
5100	3 use			275	.175		1.67	7.30			8.97	13.10
5150	4 use			280	.171		1.36	7.20			8.56	12.55

03 11 13.25 Forms In Place, Columns

		R031113-40										
0500	Round fiberglass, 4 use per mo., rent, 12" diameter		C-1	160	.200	L.F.	8.15	8.20			16.35	21.50
0550	16" diameter	R031113-60		150	.213		9.70	8.70			18.40	24
0600	18" diameter			140	.229		10.85	9.35			20.20	26.50
0650	24" diameter			135	.237		13.50	9.70			23.20	29.50
0700	28" diameter			130	.246		15.10	10.05			25.15	32
0800	30" diameter			125	.256		15.75	10.45			26.20	33.50
0850	36" diameter			120	.267		21	10.90			31.90	39.50
1500	Round fiber tube, recycled paper, 1 use, 8" diameter		G	155	.206		1.42	8.45			9.87	14.50
1550	10" diameter		G	155	.206		1.97	8.45			10.42	15.10
1600	12" diameter		G	150	.213		2.30	8.70			11	15.95
1650	14" diameter		G	145	.221		3.33	9			12.33	17.50
1700	16" diameter		G	140	.229		4.06	9.35			13.41	18.80
1720	18" diameter		G	140	.229		4.75	9.35			14.10	19.60
1750	20" diameter		G	135	.237		6.55	9.70			16.25	22
1800	24" diameter		G	130	.246		8.40	10.05			18.45	24.50
1850	30" diameter		G	125	.256		11.40	10.45			21.85	28.50
1900	36" diameter		G	115	.278		15.05	11.40			26.45	34
1950	42" diameter		G	100	.320		38	13.10			51.10	61.50
2000	48" diameter		G	85	.376		44	15.40			59.40	72
2200	For seamless type, add						15%					
3000	Round, steel, 4 use per mo., rent, regular duty, 14" diameter		G	C-1	145	.221	L.F.	15.15	9		24.15	30.50
3050	16" diameter		G		125	.256		15.50	10.45		25.95	33
3100	Heavy duty, 20" diameter		G		105	.305		17	12.45		29.45	38
3150	24" diameter		G		85	.376		18.65	15.40		34.05	44
3200	30" diameter		G		70	.457		21.50	18.70		40.20	52
3250	36" diameter		G		60	.533		23	22		45	59
3300	48" diameter		G		50	.640		68	26		94	115
3350	60" diameter		G		45	.711		42	29		71	90.50
4500	For second and succeeding months, deduct						50%					
5000	Job-built plywood, 8" x 8" columns, 1 use		C-1	165	.194	SFCA	2.31	7.95			10.26	14.70
5050	2 use			195	.164		1.32	6.70			8.02	11.75
5100	3 use			210	.152		.92	6.25			7.17	10.55
5150	4 use			215	.149		.76	6.10			6.86	10.20

655 LF
57 LF

03 11 Concrete Forming

03 11 13 – Structural Cast-In-Place Concrete Forming

03 11 13.25 Forms In Place, Columns		Crew	Daily Output	Labor-Hours	Unit	Material	2011 Bare Costs	Labor	Equipment	Total	Total Incl O&G
5500	12" x 12" columns, 1 use	C-1	180	.178	SFCA	2.19	7.25			9.44	13.55
5550	2 use		210	.152		1.20	6.25			7.45	10.85
5600	3 use		220	.145		.87	5.95			6.82	10.05
5650	4 use		225	.142		.71	5.80			6.51	9.70
6000	16" x 16" columns, 1 use		185	.173		2.17	7.05			9.22	13.25
6050	2 use		215	.149		1.16	6.10			7.26	10.45
6100	3 use		230	.139		.87	5.70			6.57	9.65
6150	4 use		235	.136		.71	5.55			6.26	9.35
6500	24" x 24" columns, 1 use		190	.168		2.43	6.90			9.33	13.20
6550	2 use		216	.148		1.33	6.05			7.38	10.75
6600	3 use		230	.139		.97	5.70			6.67	9.75
6650	4 use		238	.134		.79	5.50			6.29	9.30
7000	36" x 36" columns, 1 use		200	.160		1.77	6.55			8.32	12
7050	2 use		230	.139		1.01	5.70			6.71	9.80
7100	3 use		245	.131		.71	5.35			6.06	9
7150	4 use		250	.128		.58	5.25			5.83	8.70
7400	Steel framed plywood, based on 50 uses of purchased forms, and 4 uses of bracing lumber										
7500	8" x 8" column	C-1	340	.094	SFCA	1.81	3.85			5.66	7.90
7550	10" x 10"		350	.091		1.58	3.74			5.32	7.50
7600	12" x 12"		370	.086		1.35	3.54			4.89	6.90
7650	16" x 16"		400	.080		1.05	3.27			4.32	6.15
7700	20" x 20"		420	.076		.93	3.11			4.04	5.80
7750	24" x 24"		440	.073		.67	2.97			3.64	5.30
7755	30" x 30"		440	.073		.86	2.97			3.83	5.50
7760	36" x 36"		460	.070		.75	2.84			3.59	5.20

03 11 13.30 Forms In Place, Culvert

0010	FORMS IN PLACE, CULVERT	R031113-40	C-1	.170	.188	SFCA	3.10	7.70		10.80	15.20
0015	5' to 8' square or rectangular, 1 use			180	.178		1.83	7.25		9.08	13.15
0050	2 use	R031113-60		190	.168		1.40	6.90		8.30	12.10
0100	3 use			200	.160		1.19	6.55		7.74	11.35
0150	4 use										

03 11 13.35 Forms In Place, Elevated Slabs

0010	FORMS IN PLACE, ELEVATED SLABS	R031113-40	C-2	.470	.102	S.F.	3.16	4.28		7.44	10.05
1000	Flat plate, job-built plywood, to 15' high, 1 use	R031113-60		520	.092		1.74	3.87		5.61	7.85
1050	2 use			545	.088		1.26	3.69		4.95	7.05
1100	3 use			560	.086		1.03	3.59		4.62	6.65
1150	4 use			495	.097		1.03	4.07		5.10	7.40
1500	15' to 20' high ceilings, 4 use			450	.107		1.29	4.47		5.76	8.25
1600	21' to 35' high ceilings, 4 use			449	.107		3.59	4.48		8.07	10.85
2000	Flat slab, drop panels, job-built plywood, to 15' high, 1 use			509	.094		1.97	3.95		5.92	8.20
2050	2 use			532	.090		1.43	3.78		5.21	7.40
2100	3 use			544	.088		1.17	3.70		4.87	7
2150	4 use			480	.100		2.08	4.19		6.27	8.75
2250	15' to 20' high ceilings, 4 use			435	.110		2.34	4.63		6.97	9.70
2350	20' to 35' high ceilings, 4 use			485	.099		1.99	4.15		6.14	8.55
3000	Floor slab hung from steel beams, 1 use			535	.090		1.54	3.76		5.30	7.45
3050	2 use			550	.087		1.40	3.66		5.06	7.15
3100	3 use			565	.085		1.32	3.56		4.88	6.90
3150	4 use			415	.116		5.35	4.85		10.20	13.35
3500	Floor slab, with 1-way joist pans, 1 use			445	.108		3.73	4.52		8.25	11.05
3550	2 use										

03 11 Concrete Forming

03 11 13 – Structural Cast-In-Place Concrete Forming

03 11 13.35 Forms In Place, Elevated Slabs

			Crew	Daily Output	Labor-Hours	Unit	Material	2011 Bare Costs	Total	Total Incl O&P
							Labor	Equipment		
3.50	3600	3 use		C-2	.475	.101	S.F.	3.19	4.24	7.43
0.83	3650	4 use			500	.096		2.92	4.03	6.95
0.05	4500	With 2-way waffle domes, 1 use			405	.119		5.50	4.97	10.47
9.70	4520	2 use			450	.107		3.86	4.47	8.33
3.25	4530	3 use			460	.104		3.32	4.38	7.70
1.65	4550	4 use			470	.102		3.05	4.28	7.33
2.65	5000	Box out for slab openings, over 16" deep, 1 use			190	.253	SFCA	3.51	10.60	14.11
2.35	5050	2 use			240	.200	"	1.93	8.40	10.33
3.20	5500	Shallow slab box outs, to 10 S.F.			42	1.143	Ea.	9.65	48	57.65
1.75	5550	Over 10 S.F. (use perimeter)			600	.080	L.F.	1.29	3.35	4.64
1.30	6000	Bulkhead forms for slab, with keyway, 1 use, 2 piece			500	.096		1.67	4.03	5.70
6.100	6100	3 piece (see also edge forms)			460	.104		1.80	4.38	6.18
6.200	6200	Slab bulkhead form, 4-1/2" high, exp metal, w/keyway & stakes	G	C-1	1200	.027		1.64	1.09	2.73
6.210	6210	5-1/2" high	G		1100	.029		1.90	1.19	3.09
6.215	6215	7-1/2" high	G		960	.033		2.31	1.36	3.67
6.220	6220	9-1/2" high	G		840	.038		2.40	1.56	3.96
6.500	6500	Curb forms, wood, 6" to 12" high, on elevated slabs, 1 use			180	.178	SFCA	1.34	7.25	8.59
6.550	6550	2 use			205	.156		.74	6.40	7.14
6.600	6600	3 use			220	.145		.54	5.95	6.49
6.650	6650	4 use			225	.142		.44	5.80	6.24
6.700	7000	Edge forms to 6" high, on elevated slab, 4 use			500	.064	L.F.	.17	2.62	2.79
6.750	7500	Depressed area forms to 12" high, 4 use			300	.107		.78	4.36	5.14
6.800	7550	12" to 24" high, 4 use			175	.183		1.06	7.50	8.56
6.850	8000	Perimeter deck and rail for elevated slabs, straight			90	.356		9.70	14.55	24.25
6.900	8050	Curved			65	.492		13.35	20	33.35
6.950	8500	Void forms, round fiber, 3" diameter	G		450	.071		.99	2.91	3.90
7.000	8550	4" diameter	G		425	.075		1.46	3.08	4.54
7.050	8600	6" diameter	G		400	.080		2.28	3.27	5.55
7.100	8650	8" diameter	G		375	.085		3.90	3.49	7.39
7.150	8700	10" diameter	G		350	.091		2.52	3.74	6.26
7.200	8750	12" diameter	G		300	.107		3.20	4.36	7.56

03 11 13.40 Forms In Place, Equipment Foundations

0010	FORMS IN PLACE, EQUIPMENT FOUNDATIONS	R031113-40								
0020	1 use		C-2	160	.300	SFCA	2.66	12.60	15.26	22
0050	2 use	R031113-60		190	.253		1.47	10.60	12.07	17.85
0100	3 use			200	.240		1.07	10.05	11.12	16.60
0150	4 use			205	.234		.87	9.80	10.67	16

03 11 13.45 Forms In Place, Footings

0010	FORMS IN PLACE, FOOTINGS	R031113-40								
0020	Continuous wall, plywood, 1 use		C-1	375	.085	SFCA	5	3.49	8.49	10.85
0050	2 use	R031113-60		440	.073		2.76	2.97	5.73	7.60
0100	3 use			470	.068		2	2.78	4.78	6.45
0150	4 use			485	.066		1.63	2.70	4.33	5.95
0500	Dowel supports for footings or beams, 1 use			500	.064	L.F.	.74	2.62	3.36	4.83
1000	Integral starter wall, to 4" high, 1 use			400	.080		.79	3.27	4.06	5.85
1500	Keyway, 4 use, tapered wood, 2" x 4"	1 Carp	530	.015			.18	.65	.83	1.20
1550	2" x 6"			500	.016		.26	.69	.95	1.35
2000	Tapered plastic			530	.015		.45	.65	1.10	1.50
2250	For keyway hung from supports, add			150	.053		.74	2.30	3.04	4.34
3000	Pile cap, square or rectangular, job-built plywood, 1 use	C-1	290	.110	SFCA		2.33	4.51	6.84	9.45
3050	2 use			346	.092		1.28	3.78	5.06	7.20
3100	3 use			371	.086		.93	3.53	4.46	6.40

03 11 Concrete Forming

03 11 13 – Structural Cast-In-Place Concrete Forming

03 11 13.45 Forms In Place, Footings			Daily Output	Labor-Hours	Unit	Material	2011 Bare Costs		Total	Total Ind O&P
3150	4 use		C-1	.383	.084	SFCA	.76	3.42		4.18
4000	Triangular or hexagonal, 1 use			225	.142		2.72	5.80		8.52
4050	2 use			280	.114		1.50	4.67		6.17
4100	3 use			305	.105		1.09	4.29		5.38
4150	4 use			315	.102		.88	4.15		5.03
5000	Spread footings, job-built lumber, 1 use			305	.105		1.79	4.29		6.08
5050	2 use			371	.086		1	3.53		4.53
5100	3 use			401	.080		.72	3.26		3.98
5150	4 use			414	.077		.58	3.16		3.74
6000	Supports for dowels, plinths or templates, 2' x 2' footing			25	1.280	Ea.	4.92	52.50		57.42
6050	4' x 4' footing			22	1.455		9.85	59.50		69.35
6100	8' x 8' footing			20	1.600		19.70	65.50		85.20
6150	12' x 12' footing			17	1.882		24	77		101
7000	Plinths, job-built plywood, 1 use			250	.128	SFCA	2.49	5.25		7.74
7100	4 use			270	.119	"	.82	4.85		5.67

03 11 13.47 Forms In Place, Gas Station Forms

0010 FORMS IN PLACE, GAS STATION FORMS										
0050	Curb fascia, with template, 12 ga. steel, left in place, 9" high	G	1 Corp	50	.160	L.F.	11.80	6.90		18.70
1000	Sign or light bases, 18" diameter, 9" high	G		9	.889	Ea.	74.50	38.50		113
1050	30" diameter, 13" high	G		8	1		118	43		161
2000	Island forms, 10' long, 9" high, 3'-6" wide	G	C-1	10	3.200		330	131		461
2050	4' wide	G		9	3.556		340	145		485
2500	20' long, 9" high, 4' wide	G		6	5.333		545	218		763
2550	5' wide	G		5	6.400		570	262		832

03 11 13.50 Forms In Place, Grade Beam

0010 FORMS IN PLACE, GRADE BEAM		R031113-40								
0020	Job-built plywood, 1 use		C-2	530	.091	SFCA	2.37	3.80		6.17
0050	2 use	R031113-60		580	.083		1.30	3.47		4.77
0100	3 use			600	.080		.95	3.35		4.30
0150	4 use			605	.079		.77	3.33		4.10

03 11 13.55 Forms In Place, Mat Foundation

0010 FORMS IN PLACE, MAT FOUNDATION		R031113-40								
0020	Job-built plywood, 1 use		C-2	290	.166	SFCA	2.44	6.95		9.39
0050	2 use	R031113-60		310	.155		.99	6.50		7.49
0100	3 use			330	.145		.65	6.10		6.75
0120	4 use			350	.137		.58	5.75		6.33

03 11 13.65 Forms In Place, Slab On Grade

0010 FORMS IN PLACE, SLAB ON GRADE		R031113-40								
1000	Bulkhead forms w/keyway, wood, 6" high, 1 use		C-1	510	.063	L.F.	.86	2.57		3.43
1050	2 uses	R031113-60		400	.080		.47	3.27		3.74
1100	4 uses			350	.091		.28	3.74		4.02
1400	Bulkhead form for slab, 4-1/2" high, exp metal, incl keyway & stakes	G		1200	.027		1.64	1.09		2.73
1410	5-1/2" high	G		1100	.029		1.90	1.19		3.09
1420	7-1/2" high	G		960	.033		2.31	1.36		3.67
1430	9-1/2" high	G		840	.038		2.40	1.56		3.96
2000	Curb forms, wood, 6" to 12" high, on grade, 1 use			215	.149	SFCA	1.80	6.10		7.90
2050	2 use			250	.128		1	5.25		6.25
2100	3 use			265	.121		.72	4.94		5.66
2150	4 use			275	.116		.58	4.76		5.34
3000	Edge forms, wood, 4 use, on grade, to 6" high			600	.053	L.F.	.27	2.18		2.45
3050	7" to 12" high			435	.074	SFCA	.59	3.01		3.60
3500	For depressed slabs, 4 use, to 12" high			300	.107	L.F.	.60	4.36		4.96

03 11 Concrete Forming

03 11 13 - Structural Cast-In-Place Concrete Forming

				Daily Output	Labor-Hours	Unit	Material	2011 Bare Costs			Total	Total Incl O&P
				Crew			Labor	Equipment		Total		
		03 11 13.65 Forms In Place, Slab On Grade										
6.10	3550	To 24" high		C-1	175	.183	L.F.	.79	7.50		8.29	12.30
1.30	4000	For slab blockouts, to 12" high, 1 use			200	.160		.63	6.55		7.18	10.75
8.80	4050	To 24" high, 1 use			120	.267		.80	10.90		11.70	17.60
7.80	4100	Plastic (extruded), to 6" high, multiple use, on grade			800	.040	↓	7.90	1.64		9.54	11.20
7.30	5000	Screed, 24 ga. metal key joint, see Section 03 15 05.25										
8.55	5020	Wood, incl. wood stakes, 1" x 3"		C-1	900	.036	L.F.	.68	1.45		2.13	2.98
6.50	5050	2" x 4"			900	.036	"	.67	1.45		2.12	2.97
5.80	6000	Trench forms in floor, wood, 1 use			160	.200	SFCA	1.55	8.20		9.75	14.25
5.50	6050	2 use			175	.183		.85	7.50		8.35	12.40
5	6100	3 use			180	.178		.62	7.25		7.87	11.85
2	6150	4 use			185	.173	↓	.50	7.05		7.55	11.40
2	8760	Void form, corrugated fiberboard, 4" x 12", 4' long	G		3000	.011	S.F.	2.39	.44		2.83	3.29
4	8770	6" x 12", 4' long			3000	.011		2.81	.44		3.25	3.76
3.80	8780	1/4" thick hardboard protective cover for void form		2 Corp	1500	.011	↓	.57	.46		1.03	1.33

03 11 13.85 Forms In Place, Walls

0010	FORMS IN PLACE, WALLS	R031113-10									
0100	Box out for wall openings, to 16" thick, to 10 S.F.		C-2	24	2	Ea.	23	84		107	154
0150	Over 10 S.F. (use perimeter)	R031113-40	"	280	.171	L.F.	1.94	7.20		9.14	13.20
0250	Brick shelf, 4" w, add to wall forms, use wall area abv shelf										
0260	1 use	R031113-60	C-2	240	.200	SFCA	2.06	8.40		10.46	15.10
0300	2 use			275	.175	↓	1.13	7.30		8.43	12.50
0350	4 use			300	.160	↓	.82	6.70		7.52	11.20
0500	Bulkhead, wood with keyway, 1 use, 2 piece			265	.181	L.F.	1.69	7.60		9.29	13.50
0600	Bulkhead forms with keyway, 1 piece expanded metal, 8" wall	G	C-1	1000	.032		2.31	1.31		3.62	4.55
0610	10" wall	G		800	.040		2.40	1.64		4.04	5.15
0620	12" wall	G		525	.061	↓	2.88	2.49		5.37	7
0700	Buttress, to 8' high, 1 use		C-2	350	.137	SFCA	2.89	5.75		8.64	12
0750	2 use			430	.112		1.59	4.68		6.27	8.95
0800	3 use			460	.104		1.16	4.38		5.54	8
0850	4 use			480	.100	↓	.96	4.19		5.15	7.50
1000	Corbel or haunch, to 12" wide, add to wall forms, 1 use			150	.320	L.F.	1.91	13.40		15.31	22.50
1050	2 use			170	.282		1.05	11.85		12.90	19.30
1100	3 use			175	.274		.76	11.50		12.26	18.50
1150	4 use			180	.267	↓	.62	11.20		11.82	17.85
2000	Wall, job-built plywood, to 8' high, 1 use			370	.130	SFCA	2.17	5.45		7.62	10.75
2050	2 use			435	.110		1.38	4.63		6.01	8.60
2100	3 use			495	.097		1.01	4.07		5.08	7.35
2150	4 use			505	.095		.82	3.99		4.81	7
2400	Over 8' to 16' high, 1 use			280	.171		2.38	7.20		9.58	13.65
2450	2 use			345	.139		1.08	5.85		6.93	10.15
2500	3 use			375	.128		.77	5.35		6.12	9.10
2550	4 use			395	.122		.63	5.10		5.73	8.50
2700	Over 16' high, 1 use			235	.204		2.18	8.55		10.73	15.55
2750	2 use			290	.166		1.20	6.95		8.15	11.95
2800	3 use			315	.152		.87	6.40		7.27	10.75
2850	4 use			330	.145		.71	6.10		6.81	10.15
4000	Radial, smooth curved; job-built plywood, 1 use			245	.196		2.16	8.20		10.36	15
4050	2 use			300	.160		1.19	6.70		7.89	11.60
4100	3 use			325	.148		.86	6.20		7.06	10.45
4150	4 use			335	.143		.70	6		6.70	9.95
4200	Below grade, job-built plywood, 1 use			225	.213		2.25	8.95		11.20	16.20
4210	2 use			225	.213	↓	1.24	8.95		10.19	15.05

03 21 Reinforcing Steel

03 21 05 - Reinforcing Steel Accessories

03 21 05.75 Splicing Reinforcing Bars		Crew	Daily Output	Labor-Hours	Unit	Material	2011 Bare Costs			Total	Total Ind O&P
							Labor	Equipment	Total		
0810	#10 bars	[G]	C-5	40	1.400	Ea.	41	66	18.70	125.70	170
0900	#11 bars	[G]		32	1.750		49	82.50	23.50	155	210
0920	#14 bars	[G]		24	2.333		79.50	110	31	220.50	295
1000	Sleeve type w/ferrous filler, for critical structures, #6 bars	[G]	C-25	72	.444		41	16.80		57.80	72.50
1210	#7 bars	[G]		64	.500		42	18.90		60.90	76.50
1220	#8 bars	[G]		56	.571		44	21.50		65.50	83.50
1230	#9 bars	[G]	C-5	48	1.167		45	55	15.60	115.60	154
1240	#10 bars	[G]		40	1.400		48	66	18.70	132.70	178
1250	#11 bars	[G]		32	1.750		58	82.50	23.50	164	220
1260	#14 bars	[G]		24	2.333		73	110	31	214	288
1270	#18 bars	[G]		16	3.500		106	165	47	318	430
2000	Weldable half coupler, taper threaded, #4 bars	[G]	E-16	120	.133		7.10	6.60	.91	14.61	20.50
2100	#5 bars	[G]		112	.143		8.40	7.10	.97	16.47	23
2200	#6 bars	[G]		104	.154		13.35	7.60	1.05	22	29.50
2300	#7 bars	[G]		96	.167		15.50	8.25	1.14	24.89	33
2400	#8 bars	[G]		88	.182		16.15	9	1.24	26.39	35
2500	#9 bars	[G]		80	.200		17.80	9.90	1.36	29.06	38.50
2600	#10 bars	[G]		72	.222		18.20	11	1.51	30.71	41
2700	#11 bars	[G]		64	.250		19.45	12.40	1.70	33.55	45.50
2800	#14 bars	[G]		56	.286		22.50	14.15	1.95	38.60	51.50
2900	#18 bars	[G]		48	.333		36.50	16.50	2.27	55.27	71.50

03 21 10 - Uncoated Reinforcing Steel

03 21 10.60 Reinforcing In Place

0015	REINFORCING IN PLACE, 50-60 ton lots, A615 Grade 60	R032110-10									
0020	Includes labor, but not material cost, to install accessories										
0030	Made from recycled materials										
0100	Beams & Girders, #3 to #7										
0150	#8 to #18	R032110-20	[G]	4 Rodm	1.60	20	Ton	900	970	1,870	2,550
0200	Columns, #3 to #7		[G]		2.70	11.852		900	575	1,475	1,900
0250	#8 to #18		[G]		1.50	21.333		900	1,025	1,925	2,650
0300	Spirals, hot rolled, 8" to 15" diameter		[G]		2.30	13.913		900	675	1,575	2,075
0320	15" to 24" diameter	R032110-40	[G]		2.20	14.545		1,400	705	2,105	2,675
0330	24" to 36" diameter		[G]		2.20	14.545		1,350	705	2,055	2,600
0340	36" to 48" diameter	R032110-50	[G]		2.30	13.913		1,275	675	1,950	2,475
0360	48" to 64" diameter		[G]		2.40	13.333		1,225	645	1,870	2,350
0380	64" to 84" diameter	R032110-70	[G]		2.50	12.800		1,350	620	1,970	2,475
0390	84" to 96" diameter		[G]		2.60	12.308		1,400	595	1,995	2,500
0400	Elevated slabs, #4 to #7	R032110-80	[G]		2.70	11.852		1,475	575	2,050	2,550
0500	Footings, #4 to #7		[G]		2.90	11.034		955	535	1,490	1,900
0550	#8 to #18	Pile Caps, Caissons	[G]		2.10	15.238		855	740	1,595	2,125
0600	Slab on grade, #3 to #7		[G]		3.60	8.889		810	430	1,240	1,575
0700	Walls, #3 to #7		[G]		2.30	13.913		855	675	1,530	2,025
0750	#8 to #18		[G]		3	10.667		855	515	1,370	1,775
0900	For other than 50 - 60 ton lots				4	8		855	385	1,240	1,550
1000	Under 10 ton job, #3 to #7, add						Ton	25%	10%		
1010	#8 to #18, add							20%	10%		
1050	10 - 50 ton job, #3 to #7, add							10%			
1060	#8 to #18, add							5%			
1100	60 - 100 ton job, #3 to #7, deduct							5%			
1110	#8 to #18, deduct							10%			
1150	Over 100 ton job, #3 to #7, deduct							10%			
1160	#8 to #18, deduct							15%			

03 22 Welded Wire Fabric Reinforcing

03 22 05 – Uncoated Welded Wire Fabric

03 22 05.50 Welded Wire Fabric			Crew	Daily Output	Labor-Hours	Unit	Material	2011 Bare Costs	Equipment	Total	Total Incl Q&P
0010	WELDED WIRE FABRIC ASTM A185	R032205-30									
0030	Made from recycled materials		G								
0050	Sheets										
0100	6 x 6 - W1.4 x W1.4 (10 x 10) 21 lb. per C.S.F.		G	2 Rodm	.457	C.S.F.	12.50	22		34.50	49.50
0200	6 x 6 - W2.1 x W2.1 (8 x 8) 30 lb. per C.S.F.		G		.516		18.90	25		43.90	61
0300	6 x 6 - W2.9 x W2.9 (6 x 6) 42 lb. per C.S.F.		G		.552		21.50	26.50		48	66
0400	6 x 6 - W4 x W4 (4 x 4) 58 lb. per C.S.F.		G		.593		29.50	28.50		58	78.50
0500	4 x 4 - W1.4 x W1.4 (10 x 10) 31 lb. per C.S.F.		G		.516		17.90	25		42.90	59.50
0600	4 x 4 - W2.1 x W2.1 (8 x 8) 44 lb. per C.S.F.		G		.552		22.50	26.50		49	67
0650	4 x 4 - W2.9 x W2.9 (6 x 6) 61 lb. per C.S.F.		G		.593		36	28.50		64.50	85.50
0700	4 x 4 - W4 x W4 (4 x 4) 85 lb. per C.S.F.		G		.640		45	31		76	99
0750	Rolls										
0800	2 x 2 - #14 galv., 21 lb./C.S.F., beam & column wrap		G	2 Rodm	2.462	C.S.F.	44	119		163	239
0900	2 x 2 - #12 galv. for gunite reinforcing		G	"	2.462	"	65	119		184	262

03 23 Stressing Tendons

03 23 05 – Prestressing Tendons

03 23 05.50 Prestressing Steel

03 23 05.50 Prestressing Steel			R034136-90									
0010	PRESTRESSING STEEL											
0100	Grouted strand, post-tensioned in field, 50' span, 100 kip		G	C-3	1200	.053	lb.	2.28	2.38	.09	4.75	6.35
0150	300 kip		G		2700	.024		1.04	1.06	.04	2.14	2.85
0300	100' span, 100 kip		G		1700	.038		2.29	1.68	.06	4.03	5.20
0350	300 kip		G		3200	.020		1.98	.89	.03	2.90	3.62
0500	200' span, 100 kip		G		2700	.024		2.28	1.06	.04	3.38	4.21
0550	300 kip		G		3500	.018		1.98	.81	.03	2.82	3.48
0800	Grouted bars, 50' span, 42 kip		G		2600	.025		.91	1.10	.04	2.05	2.77
0850	143 kip		G		3200	.020		.88	.89	.03	1.80	2.40
1000	75' span, 42 kip		G		3200	.020		.93	.89	.03	1.85	2.46
1050	143 kip		G		4200	.015		.78	.68	.03	1.49	1.96
1200	Ungrouted strand, 50' span, 100 kip		G	C-4	1275	.025		.62	1.23	.02	1.87	2.67
1250	300 kip		G		1475	.022		.62	1.06	.02	1.70	2.40
1400	100' span, 100 kip		G		1500	.021		.62	1.04	.02	1.68	2.37
1450	300 kip		G		1650	.019		.62	.95	.02	1.59	2.22
1600	200' span, 100 kip		G		1500	.021		.62	1.04	.02	1.68	2.37
1650	300 kip		G		1700	.019		.62	.92	.02	1.56	2.17
1800	Ungrouted bars, 50' span, 42 kip		G		1400	.023		.45	1.12	.02	1.59	2.31
1850	143 kip		G		1700	.019		.45	.92	.02	1.39	1.99
2000	75' span, 42 kip		G		1800	.018		.45	.87	.02	1.34	1.91
2050	* 143 kip		G		2200	.015		.45	.71	.01	1.17	1.65
2220	Ungrouted single strand, 100' slab, 25 kip		G		1200	.027		.62	1.30	.02	1.94	2.79
2250	35 kip		G		1475	.022		.62	1.06	.02	1.70	2.40
3000	Slabs on grade, 0.5-inch diam. non-bonded strands, HDPE sheathed, attached dead-end anchors, loose stressing-end anchors											
3050	25' x 30' slab, strands @ 36" O.C., placing		2 Rodm	2940	.005	S.F.	.59	.26		.85	1.07	
3105	Stressing		C-4A	3750	.004			.21	.01	.22	.34	
3110	42" O.C., placing		2 Rodm	3200	.005		.52	.24		.76	.96	
3115	Stressing		C-4A	4040	.004			.19	.01	.20	.32	
3120	48" O.C., placing		2 Rodm	3510	.005		.46	.22		.68	.85	
3125	Stressing		C-4A	4390	.004			.18	.01	.19	.29	
3150	25' x 40' slab, strands @ 36" O.C., placing		2 Rodm	3370	.005		.57	.23		.80	1	
3155	Stressing		C-4A	4360	.004			.18	.01	.19	.29	

03 30 Cast-In-Place Concrete

03 30 53 – Miscellaneous Cast-In-Place Concrete

03 30 53.40 Concrete In Place		Crew	Daily Output	Labor-Hours	Unit	Material	2011 Labor	Bare Costs	Equipment	Total	Total Ind O&P
5500	Lightweight, ready mix, including screed finish only,										
5510	not including forms or reinforcing										
5550	1:4 (2500 psi) for structural roof decks	C-14B	260	.800	C.Y.	136	34.50	2.94	173.44	205	01
5600	1:6 (3000 psi) for ground slab with radiant heat	C-14F	92	.783		133	30.50	.25	163.75	192	01
5650	1:3:2 (2000 psi) with sand aggregate, roof deck	C-14B	260	.800		133	34.50	2.94	170.44	202	01
5700	Ground slab (2000 psi)	C-14F	107	.673		133	26	.22	159.22	185	01
5900	Pile caps (3000 psi), incl. forms and reinf., sq. or rect., under 10 C.Y.	C-14C	54.14	2.069		154	85.50	.43	239.93	300	01
5950	Over 10 C.Y.		75	1.493		147	61.50	.31	208.81	256	01
6000	Triangular or hexagonal, under 10 C.Y.		53	2.113		118	87.50	.44	205.94	264	01
6050	Over 10 C.Y.		85	1.318		132	54.50	.28	186.78	229	02
6200	Retaining walls (3000 psi), gravity, 4' high see Section 32 32	C-14D	66.20	3.021		136	130	11.55	277.55	360	04
6250	10' high		125	1.600		129	68.50	6.10	203.60	254	04
6300	Cantilever, level backfill loading, 8' high		70	2.857		148	122	10.90	280.90	365	04
6350	16' high		91	2.198		141	94	8.40	243.40	310	06
6800	Stairs (3500 psi), not including safety treads, free standing, 3'-6" wide	C-14H	83	.578	LF Nose	5	24.50	.28	29.78	43.50	08
6850	Cast on ground		125	.384	"	4.28	16.30	.18	20.76	30	08
7000	Stair landings, free standing		200	.240	S.F.	4.03	10.20	.12	14.35	20	10
7050	Cast on ground		475	.101	"	3.29	4.29	.05	7.63	10.25	10

03 31 Structural Concrete

03 31 05 – Normal Weight Structural Concrete

03 31 05.30 Concrete, Field Mix

0010	CONCRETE, FIELD MIX	R033105-65			C.Y.	85			85	93.50
0015	FOB forms 2250 psi				"	92			92	101
0020	3000 psi									

03 31 05.35 Normal Weight Concrete, Ready Mix

0010	NORMAL WEIGHT CONCRETE, READY MIX, delivered	R033105-10								
0012	Includes local aggregate, sand, Portland cement, and water									
0015	Excludes all additives and treatments	R033105-20			C.Y.	91.50			91.50	101
0020	2000 psi									
0100	2500 psi	R033105-30				94			94	103
0150	3000 psi					99			99	109
0200	3500 psi	R033105-40				99.50			99.50	110
0300	4000 psi					103			103	113
0350	4500 psi	R033105-50				106			106	116
0400	5000 psi					111			111	122
0411	6000 psi					127			127	139
0412	8000 psi					206			206	227
0413	10,000 psi					293			293	320
0414	12,000 psi					355			355	390
1000	For high early strength cement, add					10%				
1010	For structural lightweight with regular sand, add					25%				
1300	For winter concrete (hot water), add					4.25			4.25	4.68
1400	For hot weather concrete (ice), add					9.35			9.35	10.25
1410	For mid-range water reducer, add					4.13			4.13	4.54
1420	For high-range water reducer/superplasticizer, add					6.35			6.35	6.95
1430	For retarder, add					2.71			2.71	2.98
1440	For non-Chloride accelerator, add					4.83			4.83	5.30
1450	For Chloride accelerator, per 1%, add					3.28			3.28	3.61
1460	For fiber reinforcing, synthetic (1 lb./C.Y.), add					6.65			6.65	7.30
1500	For Saturday delivery, add					8.85			8.85	9.70

03 31 Structural Concrete

03 31 05 – Normal Weight Structural Concrete

03 31 05.35 Normal Weight Concrete, Ready Mix

		Crew	Daily Output	Labor-Hours	Unit	Material	2011 Bare Costs		Total	Total Incl O&P
						Hr.	Labor	Equipment		
						Ea.				
1510	For truck holding/waiting time past 1st hour per load, add								87.50	96.50
1520	For short load (less than 4 C.Y.), add per load								112	124
2000	For all lightweight aggregate, add								45%	

03 31 05.70 Placing Concrete

0010	PLACING CONCRETE	R033105-70								
0020	Includes labor and equipment to place, strike off and consolidate									
0050	Beams, elevated, small beams, pumped									
0100	With crane and bucket									
0200	Large beams, pumped									
0250	With crane and bucket									
0400	Columns, square or round, 12" thick, pumped									
0450	With crane and bucket									
0600	18" thick, pumped									
0650	With crane and bucket									
0800	24" thick, pumped									
0850	With crane and bucket									
1000	36" thick, pumped									
1050	With crane and bucket									
1400	Elevated slabs, less than 6" thick, pumped									
1450	With crane and bucket									
1500	6" to 10" thick, pumped									
1550	With crane and bucket									
1600	Slabs over 10" thick, pumped									
1650	With crane and bucket									
1900	Footings, continuous, shallow, direct chute									
1950	Pumped									
2000	With crane and bucket									
2100	Footings, continuous, deep, direct chute	<i>caissons</i>								
2150	Pumped									
2200	With crane and bucket									
2400	Footings, spread, under 1 C.Y., direct chute									
2450	Pumped									
2500	With crane and bucket									
2600	Over 5 C.Y., direct chute									
2650	Pumped									
2700	With crane and bucket									
2900	Foundation mats, over 20 C.Y., direct chute									
2950	Pumped									
3000	With crane and bucket									
3200	Grade beams, direct chute									
3250	Pumped									
3300	With crane and bucket									
3500	High rise, for more than 5 stories, pumped, add per story									
3510	With crane and bucket, add per story									
3700	Pile caps, under 5 C.Y., direct chute									
3750	Pumped									
3800	With crane and bucket									
3850	Pile cap, 5 C.Y. to 10 C.Y., direct chute									
3900	Pumped									
3950	With crane and bucket									
4000	Over 10 C.Y., direct chute									
4050	Pumped									

03 31 Structural Concrete

03 31 05 – Normal Weight Structural Concrete

03 31 05.70 Placing Concrete	Crew	Daily Output	Labor-Hours	Unit	Material	2011 Bare Costs			Total	Total Incl O&P
						Labor	Equipment	Total		
4100 With crane and bucket	C-7	185	.389	C.Y.		14.45	6.90	21.35	29.50	
4300 Slob on grade, up to 6" thick, direct chute	C-6	110	.436			15.60	.42	16.02	24	
4350 Pumped	C-20	130	.492			18.10	6.05	24.15	34	
4400 With crane and bucket	C-7	110	.655			24.50	11.60	36.10	50	
4600 Over 6" thick, direct chute	C-6	165	.291			10.40	.28	10.68	16.10	
4650 Pumped	C-20	185	.346			12.75	4.26	17.01	24	
4700 With crane and bucket	C-7	145	.497			18.45	8.80	27.25	37.50	
4900 Walls, 8" thick, direct chute	C-6	90	.533			19.10	.52	19.62	29.50	
4950 Pumped	C-20	100	.640			23.50	7.85	31.35	44	
5000 With crane and bucket	C-7	80	.900			33.50	15.95	49.45	68	
5050 12" thick, direct chute	C-6	100	.480			17.15	.47	17.62	26.50	
5100 Pumped	C-20	110	.582			21.50	7.15	28.65	40.50	
5200 With crane and bucket	C-7	90	.800			29.50	14.20	43.70	60.50	
5300 15" thick, direct chute	C-6	105	.457			16.35	.44	16.79	25.50	
5350 Pumped	C-20	120	.533			19.60	6.55	26.15	37	
5400 With crane and bucket	C-7	95	.758			28	13.45	41.45	57.50	
5600 Wheeled concrete dumping, add to placing costs above										
5610 Walking cart, 50' haul, odd	C-18	32	.281	C.Y.		9.70	1.72	11.42	16.80	
5620 150' haul, odd		24	.375			12.95	2.29	15.24	22.50	
5700 250' haul, odd		18	.500			17.30	3.06	20.36	30	
5800 Riding cart, 50' haul, odd	C-19	80	.113			3.89	1.15	5.04	7.20	
5810 150' haul, odd		60	.150			5.20	1.53	6.73	9.65	
5900 250' haul, odd		45	.200			6.90	2.04	8.94	12.85	

03 35 Concrete Finishing

03 35 29 – Toolled Concrete Finishing

03 35 29.30 Finishing Floors

0010 FINISHING FLOORS									
0012 Finishing requires that concrete first be placed, struck off & consolidated									
0015 Basic finishing for various unspecified flatwork									
0100 Bull float only	C-10	4000	.006	S.F.		.23		.23	.34
0125 Bull float & manual float		2000	.012			.46		.46	.69
0150 Bull float, manual float, & broom finish, w/edging & joints		1850	.013			.50		.50	.74
0200 Bull float, manual float & manual steel trowel		1265	.019			.73		.73	1.09
0210 For specified Random Access Floors in ACI Classes 1, 2, 3 and 4 to achieve Composite Overall Floor Flatness and Levelness values up to F35/F25									
0250 Bull float, machine float & machine trowel (walk-behind)	C-10C	1715	.014	S.F.		.54	.02	.56	.82
0300 Power screed, bull float, machine float & trowel (walk-behind)	C-10D	2400	.010			.39	.04	.43	.62
0350 Power screed, bull float, machine float & trowel (ride-on)	C-10E	4000	.006			.23	.06	.29	.40
0352 For specified Random Access Floors in ACI Classes 5, 6, 7 and 8 to achieve Composite Overall Floor Flatness and Levelness values up to F50/F50									
0354 Add for two-dimensional straightening after power float	C-10	6000	.004	S.F.		.15		.15	.23
0358 For specified Random or Defined Access Floors in ACI Class 9 to achieve Composite Overall Floor Flatness and Levelness values up to F100/F100									
0360 Add for two-dimensional straightening after bull float & power float	C-10	3000	.008	S.F.		.31		.31	.46
0362 For specified Superflat Defined Access Floors in ACI Class 9 to achieve Minimum Floor Flatness and Levelness values of F100/F100									
0368 Add for 2-dim'l straightening after bull float, power float, power trowel	C-10	2000	.012	S.F.		.46		.46	.69
0400 Integral topping and finish, using 1:1:2 mix, 3/16" thick	C-10B	1000	.040		.10	1.48	.23	1.81	2.58
0450 1/2" thick		950	.042			.27	1.56	.24	2.07
0500 3/4" thick		850	.047			.41	1.74	.27	2.42

05 12 Structural Steel Framing

05 12 23 – Structural Steel for Buildings

05 12 23.75 Structural Steel Members		Crew	Daily Output	Labor-Hours	Unit	2011 Bare Costs			Total	Total Incl O&G
						Material	Labor	Equipment		
0020	Shop fab'd for 100-ton, 1-2 story project, bolted connections									
0100	W 6 x 9	G	E-2	600	.093	L.F.	11.15	4.42	2.70	18.27
0120	x 15	G		600	.093		18.55	4.42	2.70	25.67
0140	x 20	G		600	.093		25	4.42	2.70	32.12
0300	W 8 x 10	G		600	.093		12.40	4.42	2.70	19.52
0320	x 15	G		600	.093		18.55	4.42	2.70	25.67
0350	x 21	G		600	.093		26	4.42	2.70	33.12
0360	x 24	G		550	.102		29.50	4.82	2.95	37.27
0370	x 28	G		550	.102		34.50	4.82	2.95	42.27
0500	x 31	G		550	.102		38.50	4.82	2.95	46.27
0520	x 35	G		550	.102		43.50	4.82	2.95	51.27
0540	x 48	G		550	.102		59.50	4.82	2.95	67.27
0600	W 10 x 12	G		600	.093		14.85	4.42	2.70	21.97
0620	x 15	G		600	.093		18.55	4.42	2.70	25.67
0700	x 22	G		600	.093		27	4.42	2.70	34.12
0720	x 26	G		600	.093		32	4.42	2.70	39.12
0740	x 33	G		550	.102		41	4.82	2.95	48.77
0900	x 49	G		550	.102		60.50	4.82	2.95	68.27
1100	W 12 x 16	G		880	.064		19.80	3.01	1.84	24.65
1300	x 22	G		880	.064		27	3.01	1.84	31.85
1500	x 26	G		880	.064		32	3.01	1.84	36.85
1520	x 35	G		810	.069		43.50	3.27	2	48.77
1560	x 50	G		750	.075		62	3.54	2.16	67.70
1580	x 58	G		750	.075		72	3.54	2.16	77.70
1700	x 72	G		640	.088		89	4.14	2.53	95.67
1740	x 87	G		640	.088		108	4.14	2.53	114.67
1900	W 14 x 26	G		990	.057		32	2.68	1.64	36.32
2100	x 30	G		900	.062		37	2.95	1.80	41.75
2300	x 34	G		810	.069		42	3.27	2	47.27
2320	x 43	G		810	.069		53	3.27	2	58.27
2340	x 53	G		800	.070		65.50	3.32	2.03	70.85
2360	x 74	G		760	.074		91.50	3.49	2.13	97.12
2380	x 90	G		740	.076		111	3.58	2.19	116.77
2500	x 120	G		720	.078		149	3.68	2.25	154.93
2700	W 16 x 26	G		1000	.056		32	2.65	1.62	36.27
2900	x 31	G		900	.062		38.50	2.95	1.80	43.25
3100	x 40	G		800	.070		49.50	3.32	2.03	54.85
3120	x 50	G		800	.070		62	3.32	2.03	67.35
3140	x 67	G		760	.074		83	3.49	2.13	88.62
3300	W 18 x 35	G	E-5	960	.083		43.50	3.99	1.80	49.29
3500	x 40	G		960	.083		49.50	3.99	1.80	55.29
3520	x 46	G		960	.083		57	3.99	1.80	63.50
3700	x 50	G		912	.088		62	4.20	1.90	68.10
3900	x 55	G		912	.088		68	4.20	1.90	74.10
3920	x 65	G		900	.089		80.50	4.26	1.92	86.68
3940	x 76	G		900	.089		94	4.26	1.92	100.18
3960	x 86	G		900	.089		106	4.26	1.92	112.18
3980	x 106	G		900	.089		131	4.26	1.92	137.18
4100	W 21 x 44	G		1064	.075		54.50	3.60	1.63	59.73
4300	x 50	G		1064	.075		62	3.60	1.63	67.23
4500	x 62	G		1036	.077		76.50	3.70	1.67	81.87
4700	x 68	G		1036	.077		84	3.70	1.67	89.37
4720	x 83	G		1000	.080		103	3.83	1.73	108.56

05 12 Structural Steel Framing

05 12 23 – Structural Steel for Buildings

				Daily Output	Labor-Hours	Unit	Material	2011 Bare Costs	Total	Total Ind O&P
				Crew			Labor	Equipment		
		05 12 23.75 Structural Steel Members								
22.50	4740	x 93	[G]	E-5	1000	.080	L.F.	115	3.83	1.73
31	4760	x 101	[G]		1000	.080		125	3.83	1.73
37.50	4780	x 122	[G]		1000	.080		151	3.83	1.73
24	4900	W 24 x 55	[G]		1110	.072		68	3.45	1.56
31	5100	x 62	[G]		1110	.072		76.50	3.45	1.56
39	5300	x 68	[G]		1110	.072		84	3.45	1.56
44	5500	x 76	[G]		1110	.072		94	3.45	1.56
49.50	5700	x 84	[G]		1080	.074		104	3.55	1.60
53.50	5720	x 94	[G]		1080	.074		116	3.55	1.60
59	5740	x 104	[G]		1050	.076		129	3.65	1.65
77	5760	x 117	[G]		1050	.076		145	3.65	1.65
27	5780	x 146	[G]		1050	.076		181	3.65	1.65
31	5800	W 27 x 84	[G]		1190	.067		104	3.22	1.45
40.50	5900	x 94	[G]		1190	.067		116	3.22	1.45
46	5920	x 114	[G]		1150	.070		141	3.33	1.51
56.50	5940	x 146	[G]		1150	.070		181	3.33	1.51
5960	5960	x 161	[G]		1150	.070		199	3.33	1.51
78	6100	W 30 x 99	[G]		1200	.067		123	3.19	1.44
29	6300	x 108	[G]		1200	.067		134	3.19	1.44
37	6500	x 116	[G]		1160	.069		144	3.31	1.49
42.50	6520	x 132	[G]		1160	.069		163	3.31	1.49
55.50	6540	x 148	[G]		1160	.069		183	3.31	1.49
76.50	6560	x 173	[G]		1120	.071		214	3.42	1.55
87.50	6580	x 191	[G]		1120	.071		236	3.42	1.55
38	6700	W 33 x 118	[G]		1176	.068		146	3.26	1.47
28	6900	x 130	[G]		1134	.071		161	3.38	1.53
42	7100	x 141	[G]		1134	.071		174	3.38	1.53
18	7120	x 169	[G]		1100	.073		209	3.49	1.57
54.50	7140	x 201	[G]		1100	.073		249	3.49	1.57
56.50	7300	W 36 x 135	[G]		1170	.068		167	3.28	1.48
10	7500	x 150	[G]		1170	.068		186	3.28	1.48
19	7600	x 170	[G]		1150	.070		210	3.33	1.51
11	7700	x 194	[G]		1125	.071		240	3.41	1.54
2	7900	x 231	[G]		1125	.071		286	3.41	1.54
2	7920	x 262	[G]		1035	.077		325	3.70	1.67
9	8100	x 302	[G]		1035	.077		375	3.70	1.67
2.50	8490	For projects 75 to 99 tons, add						10%		
6	8492	50 to 74 tons, add						20%		
9.50	8494	25 to 49 tons, add						30%	10%	
6.50	8496	10 to 24 tons, add						50%	25%	
3.50	8498	2 to 9 tons, add						75%	50%	
1.50	8499	Less than 2 tons, add						100%	100%	

05 12 23.77 Structural Steel Projects

0010	STRUCTURAL STEEL PROJECTS	R050516-30								
0015	Made from recycled materials	[G]								
0020	Shop fab'd for 100-ton, 1-2 story project, bolted connections									
0200	Apartments, nursing homes, etc., 1 to 2 stories	R050523-10	[G]	E-5	10.30	7.767	Ton	2,250	370	168
0300	3 to 6 stories		[G]	"	10.10	7.921		2,300	380	171
0400	7 to 15 stories	R051223-10	[G]	E-6	14.20	9.014		2,350	430	133
0500	Over 15 stories		[G]	"	13.90	9.209		2,425	440	136
0700	Offices, hospitals, etc., steel bearing, 1 to 2 stories	R051223-20	[G]	E-5	10.30	7.767		2,250	370	168
0800	3 to 6 stories		[G]	E-6	14.40	8.889		2,300	425	131



University Sciences Building

Northeastern U.S.

APPENDIX D - General Conditions Estimate Breakdown

GENERAL CONDITIONS
ESTIMATE

All figures were obtained from RS Means Building Construction
Data 2011

Project Duration = 22 Months
Bulk Construction = 20 Months

SUPERVISION AND PERSONNEL

Description	Quantity	Unit	Unit Rate	Cost
Vice President	95	WEEK	\$ 2,500	\$ 237,500
Project Executive	95	WEEK	\$ 2,200	\$ 209,000
Project Superintendent	95	WEEK	\$ 1,925	\$ 182,875
Assistant Superintendent	95	WEEK	\$ 1,800	\$ 171,000
Field Engineer	95	WEEK	\$ 1,265	\$ 120,175
Project Manager	95	WEEK	\$ 2,075	\$ 197,125
Project Engineer	95	WEEK	\$ 1,800	\$ 171,000
Office Engineer	95	WEEK	\$ 1,265	\$ 120,175
Project Administrator	22	MONTH	\$ 800	\$ 17,600
Safety Coordinator	95	WEEK	\$ 175	\$ 16,625
Project Scheduler	95	WEEK	\$ 225	\$ 21,375
Estimating Expenses	1	LS	\$ 45,000	\$ 45,000
TOTAL				\$ 1,509,450

CONSTRUCTION FACILITIES AND EQUIPMENT

Description	Quantity	Unit	Unit Rate	Cost
Field Office Trailer Set-up	1	LS	\$ 2,000	\$ 2,000
Field Office Trailer Rental	22	MONTH	\$ 425	\$ 9,350
Field Office Trailer Removal	1	LS	\$ 2,500	\$ 2,500
Construction Site Fence	20	MONTH	\$ 600	\$ 12,000
Sidewalk Overhead Protection	1	LS	\$ 1,250	\$ 1,250
Storage Trailer	15	MONTH	\$ 140	\$ 2,100
Gang Box	20	MONTH	\$ 55	\$ 1,100
Tools/Equipment	20	MONTH	\$ 650	\$ 13,000
Fire Extinguishers	20	MONTH	\$ 275	\$ 5,500
Copier/Fax/Printer	22	MONTH	\$ 400	\$ 8,800
Computer/LAN Equipment	22	MONTH	\$ 2,400	\$ 52,800
Mobile Phones	22	MONTH	\$ 325	\$ 7,150
Personal Protective Equipment	20	MONTH	\$ 250	\$ 5,000
Signage	1	LS	\$ 2,600	\$ 2,600
Dumpsters	20	MONTH	\$ 1,800	\$ 36,000
TOTAL				\$ 161,150

TEMPORARY UTILITIES

Description	Quantity	Unit	Unit Rate	Cost
Field IT/Network Set-up	1	LS	\$ 4,250	\$ 4,250
Temporary Power Installation	1	LS	\$ 15,000	\$ 15,000
Temporary Power Consumption	20	MONTH	\$ 750	\$ 15,000
Temporary Water/Sanitary Supply	1	LS	\$ 1,500	\$ 1,500
Temporary Toilets	22	MONTH	\$ 550	\$ 12,100
Potable Water	22	MONTH	\$ 175	\$ 3,850
TOTAL				\$ 51,700

MISCELLANEOUS COSTS

Description	Quantity	Unit	Unit Rate	Cost
Progress Photographs	20	MONTH	\$ 350	\$ 7,000
Document Reproduction	1	LS	\$ 25,000	\$ 25,000
Travel Expenses (Staff Vehicles)	20	MONTH	\$ 3,500	\$ 70,000
Delivery/Shipping Expenses	20	MONTH	\$ 300	\$ 6,000
Clean-up Expenses	20	MONTH	\$ 2,000	\$ 40,000
Misc. Field Expenses	20	MONTH	\$ 1,000	\$ 20,000
Office Supplies	20	MONTH	\$ 86	\$ 1,720
QC and Commissioning (0.5%)	1	LS	\$ 250,000	\$ 250,000
Permits (0.75%)	1	LS	\$ 375,000	\$ 375,000
Insurance (0.3%)	1	LS	\$ 150,000	\$ 150,000
Bonds (0.6%)	1	LS	\$ 300,000	\$ 300,000
TOTAL				\$ 1,244,720

GENERAL CONDITIONS SUMMARY

Description	Cost/Month	Cost
Supervision and Personnel	\$ 68,611	\$ 1,509,450
Construction Facilities and Equipment	\$ 7,325	\$ 161,150
Temporary Utilities	\$ 2,350	\$ 51,700
Miscellaneous Costs	\$ 56,578	<u>\$ 1,244,720</u>
TOTAL		\$ 2,967,020