# THE COMMONWEALTH MEDICAL COLLEGE (TCMC)









#### SENIOR THESIS 2013 ADVISOR: HEATHER SUSTERSIC

# XIAO YE ZHENG | STRUCTURAL OPTION

o Bi	uilding Introduction	<ul> <li>Medical Co</li> </ul>
$\circ$ Ex	xisting Structural System	○ 185,000 SF
o Pr	oblem Statement	o 4 Story Bui
o Pr	coposed Solution	Pentho
o No	ew Lateral Loads	o Maximum l
o La	ateral Frame Designs	• Cost \$120 M
o Fa	açade Design Breadth	<ul> <li>May 2009 t</li> </ul>
o A	cknowledgements	·
		<ul> <li>Design-Bid</li> </ul>

• Seeking LEED Silver

# **BUILDING INTRODUCTION**

#### SITE MAP

#### ollege

- ilding plus a buse Height at 102' Million
- to Oct 2011 I-Build EED Silver



Photos From TCMC



mage

from Google Map, edited by Xiao

- Building Introduction
- Existing Structural System
- Problem Statement
- Proposed Solution
- New Lateral Loads
- Lateral Frame Designs
- Façade Design Breadth
- Acknowledgements

Structural/M Constructio Landscap

Interi

#### PROJECT TEAM

Owner	]
Architects	I
I.E.P. Engineers	I
on Management	(
pe Architecture	ľ
ior Architecture	I

TCMC

Highland Associates & HOK

Highland Associates

Quandel Construction Group

McLane Associates

Highland Associates & HOK



# HIGHLAND

architecture engineering interiordesign





<ul> <li>Building Introduction</li> </ul>	<ul> <li>West Wing</li> </ul>
<ul> <li>Existing Structural System</li> </ul>	Foundation
<ul> <li>Problem Statement</li> </ul>	4'-0" thick,
<ul> <li>Proposed Solution</li> </ul>	bearing pres
<ul> <li>New Lateral Loads</li> </ul>	• Floor- com
<ul> <li>Lateral Frame Designs</li> </ul>	deck, norma
<ul> <li>Façade Design Breadth</li> </ul>	concrete top
• Acknowledgements	thick

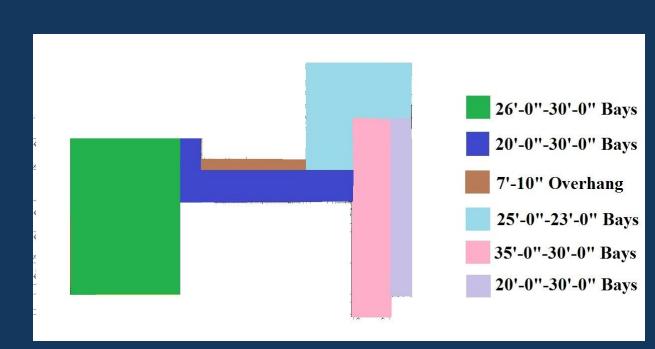
• Acknowledgements

#### EXISTING STRUCTURAL SYSTEM

• East Wing

- mat slab, , 3000 psf ssure
- posite steel al weight pping, 7.5"

- Foundation-drilled caissons, 36" to 60" in diameters, carry loads to bedrock.
- Floor- composite steel deck, lightweight concrete topping, 5.25" thick



#### BAY SIZES

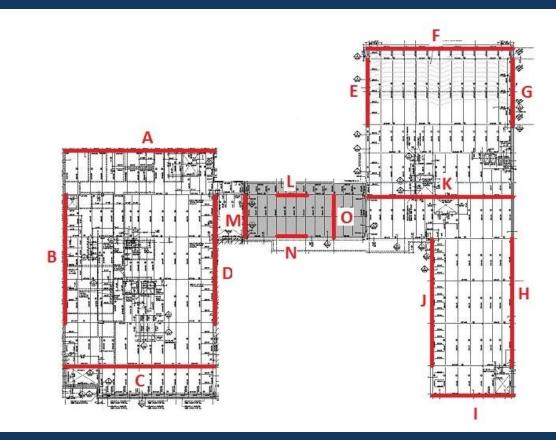
Image from Highland Associates, edited by Xiao

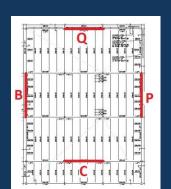
<ul> <li>Building Introduction</li> </ul>	
<ul> <li>Existing Structural System</li> </ul>	<b>.</b>
<ul> <li>Problem Statement</li> </ul>	o Framing
<ul> <li>Proposed Solution</li> </ul>	• Con
<ul> <li>New Lateral Loads</li> </ul>	• W83
<ul> <li>Lateral Frame Designs</li> </ul>	o Lateral
<ul> <li>Façade Design Breadth</li> </ul>	• 15 n
• Acknowledgements	• 13 I

#### EXISTING STRUCTURAL SYSTEM

#### MOMENT FRAMES

- ng System
- mposite steel frame
- 3x24 to W14x257, lightest to heaviest
- l System
- moment frames (not including penthouse)







#### lited by Xiao

- Building Introduction
- Existing Structural System
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- Proposed Solution
- New Lateral Loads
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#### **ROOF HEIGHTS**

#### ROOF HEIGHTS PLAN



Image from Google Map

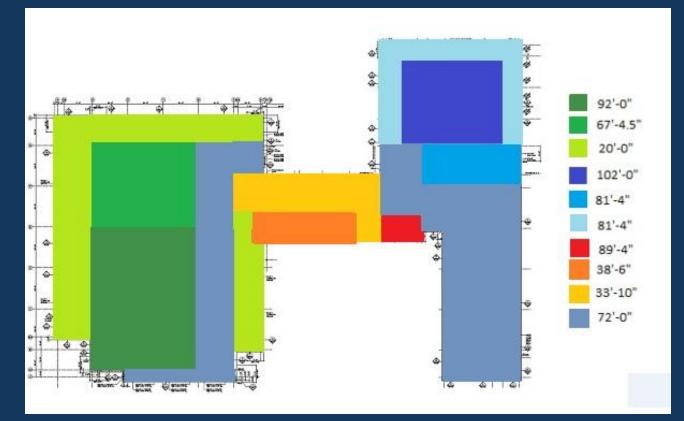
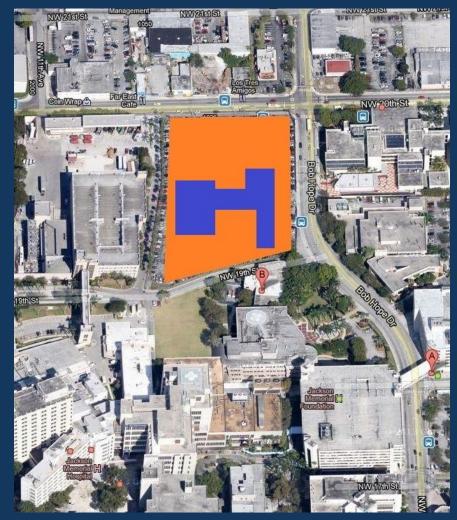


Image from Highland Associates, edited by Xiao

<ul> <li>Building Introduction</li> </ul>	o Interes
<ul> <li>Existing Structural System</li> </ul>	o Interes
<ul> <li>Problem Statement</li> </ul>	o Scenar
<ul> <li>Proposed Solution</li> </ul>	
<ul> <li>New Lateral Loads</li> </ul>	■ TC
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<ul> <li>Façade Design Breadth</li> </ul>	mp
<ul> <li>Acknowledgements</li> </ul>	■ Gee
	Da

#### PROBLEM STATEMENT

- t in Wind Design
- st in Steel Design
- rio Created for Thesis
- CMC is to be designed on a site in Miami, FL
- irricane Prone region, with wind speed up to 150 oh in building code.
- eotechnical report obtained from site in Miamiade County, Florida



#### MIAMI, FL SITE

Image from Google Map, edited by Xiao

<ul> <li>Building Introduction</li> </ul>	
<ul> <li>Existing Structural System</li> </ul>	
<ul> <li>Problem Statement</li> </ul>	o Two L
<ul> <li>Proposed Solution</li> </ul>	• Cc
<ul> <li>New Lateral Loads</li> </ul>	• Cc
<ul> <li>Lateral Frame Designs</li> </ul>	o Found
<ul> <li>Façade Design Breadth</li> </ul>	
• Acknowledgements	• M.

#### PROPOSED SOLUTION

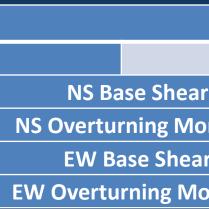
# PROJECT GOALS

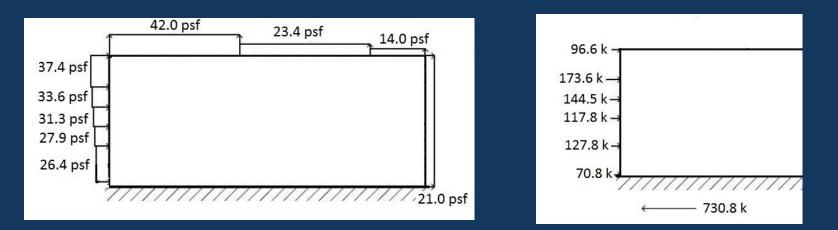
- Lateral Systems Proposed
- ode Minimum Steel Moment Frames
- ode Minimum Chevron Braced Frames
- lation
- AT Foundation

- Comparison Between Designs • Moment Frames to Braced Frames • New Systems to the Original System

- A Typical Braced Connection

- Building Introduction
- Existing Structural System
- Problem Statement
- Proposed Solution
- New Lateral Loads
- Lateral Frame Designs
- Façade Design Breadth
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Wind Pressure and Wind Force acting on West Wing, EW Direction

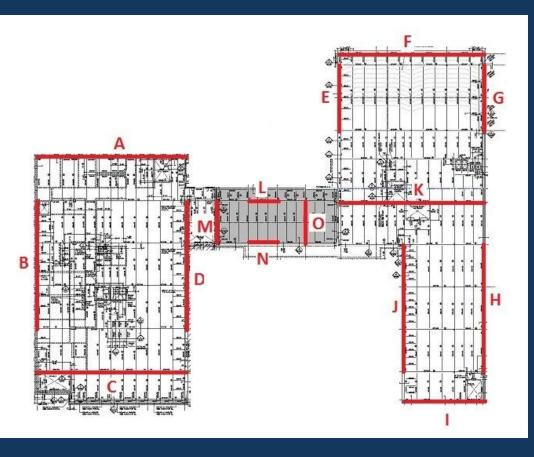
#### NEW LOADS

#### COMPARISON

Summary: Wind Loads on TCMC										
	West Wing	East Wing								
r	560.0	k	296	k						
oment	27,500.0	k-ft	14,500	k-ft						
r	731.0	k	960	k						
oment	35,800.0	k-ft	47,220	k-ft						

Comparison of Seismic and Wind Forces											
		West Wing		East Wing							
Miami, FL	Wind, N-S	Wind, E-W	Seismic	Wind, N-S	Wind, E-W	Seismic					
Base Shear (k)	560	730	136	300	970	126					
Overturning Moment (k-ft)	27,500	35,800	7,950	14,500	47,300	7,350					
Scranton, PA	Wind, N-S	Wind, E-W		Wind, N-S	Wind, E-W	Seismic					
Base Shear (k)	200	270	130	110	350	120					
Overturning Moment (k-ft)	10,000	12,900	7,600	5,230	17,100	7,000					

- Building Introduction
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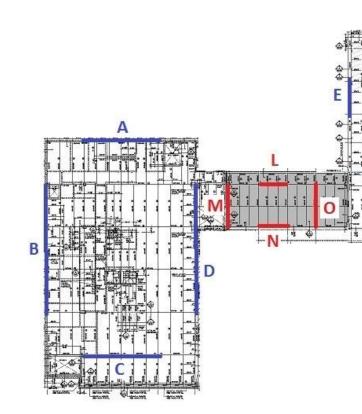


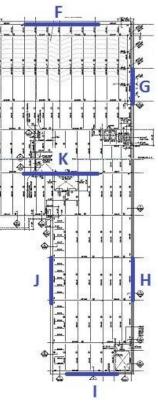
## LATERAL FRAME LAYOUTS

#### Moment Frame Layout

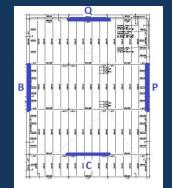
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#### Braced Frame Layout



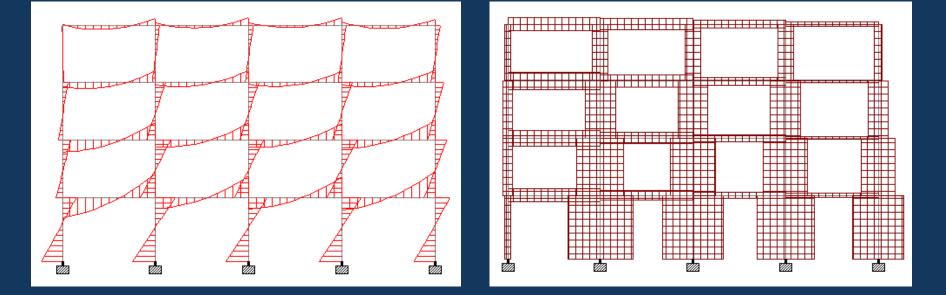


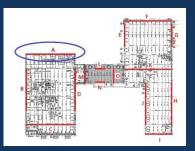
# Moment Frame Braced Frame



- Building Introduction
- Existing Structural System
- Problem Statement
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- Lateral Frame Designs
- Façade Design Breadth
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ETABS Model AE 530 Computer Modeling



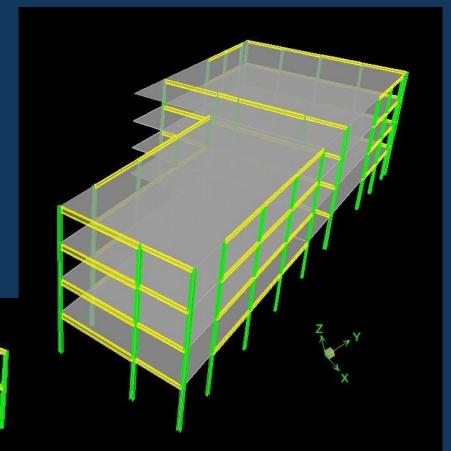


## MOMENT FRAME DESIGN

#### STAAD Model for Frame A

1.2D + 1.6W + L + 0.5Lr

#### Etabs Models





- Building Introduction
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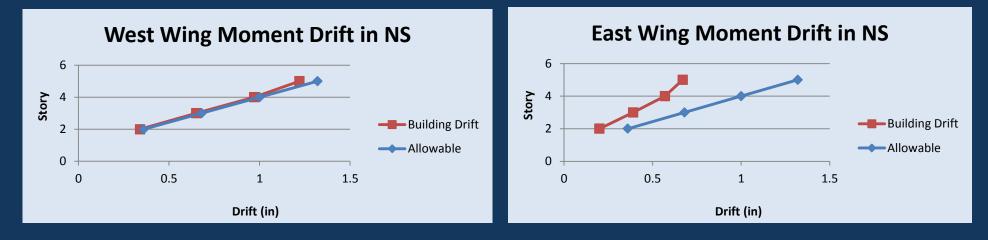


#### MOMENT FRAME DESIGN

#### Frame A Sizes

0X372		W40X372		W40X372		W40X372	
	W14X605		W14X605		W14X605		W14X605
0X372		W40X372		W40X372		W40X372	_
	W14X605		W14X605		W14X605		W14X605
0X372		W40X372		W40X372		W40X372	
	W14X605		W14X605		W14X605		W14X605
0X372		W40X372		W40X372		W40X372	
	W14X605		W14X605		W14X605		W14X605

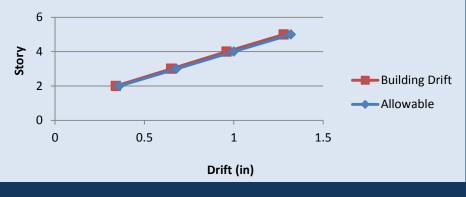
#### Moment Frame Drift



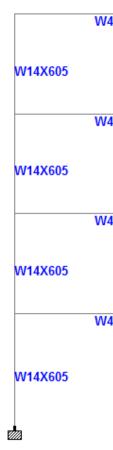
West Wing Moment Drift in EW



East Wing Moment Drift in EW



- Building Introduction
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# MOMENT FRAME DESIGN

#### Frame A Sizes

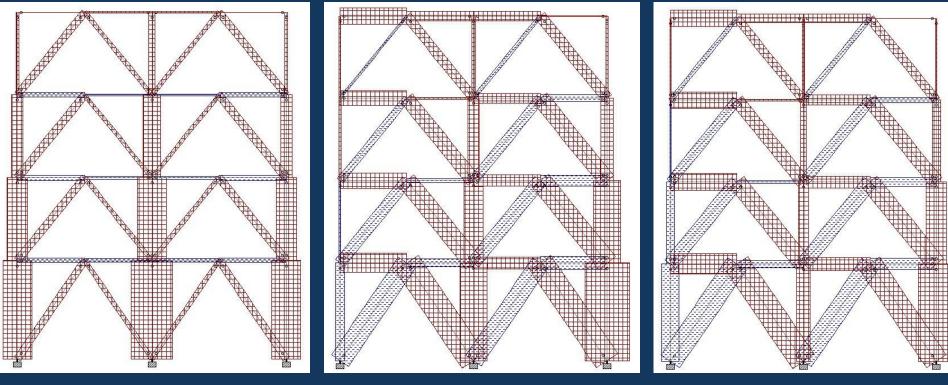
40X372		W40X372		W40X372		W40X372	
	W14X605		W14X605		W14X605		W14X605
40X372		W40X372		W40X372		W40X372	
	W14X605		W14X605		W14X605		W14X605
40X372		W40X372		W40X372		W40X372	
	W14X605		W14X605		W14X605		W14X605
40X372		W40X372		W40X372		W40X372	
	W14X605		W14X605		W14X605		W14X605

#### Original Frame A Sizes

W30X99	W	30X99	W30X99	W30X99	
W14X257	W14X257	W14X257		W14X257	W14X257
W30X99	W	30X99	W30X99	W30X99	
W14X257	W14X257	W14X257		W14X257	W14X257
W30X99	W	30X99	W30X99	W30X99	
W14X257	W14X257	W14X257		W14X257	W14X257
W30X99	W	30X99	W30X99	W30X99	
W14X257	W14X257	W14X257		W14X257	W14X257
2			F		r an

- Building Introduction
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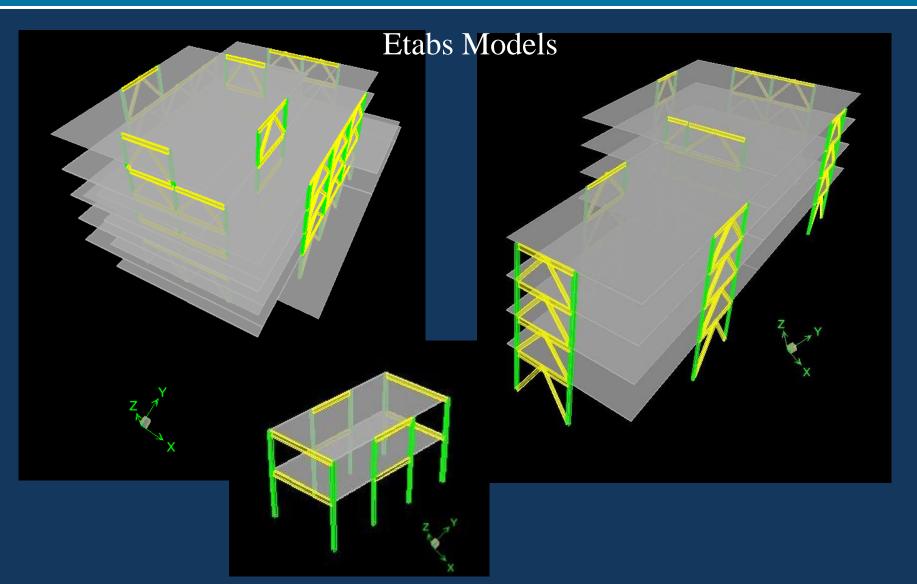
ETABS Model AE 530 Computer Modeling



1.2D + 1.6L + 0.5 Lr 1.2D + 1.6W + L + 0.5Lr

# BRACED FRAME DESIGN

#### STAAD Model for Frame A

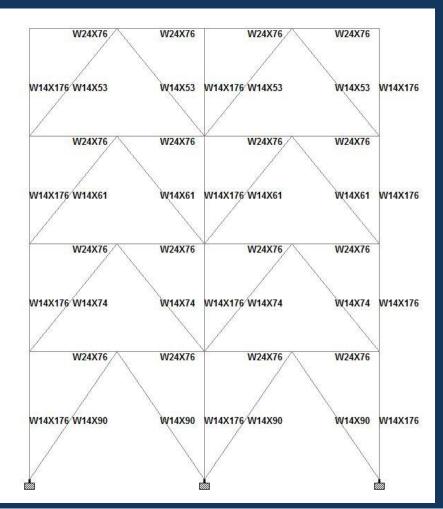


0.9D + 1.6W

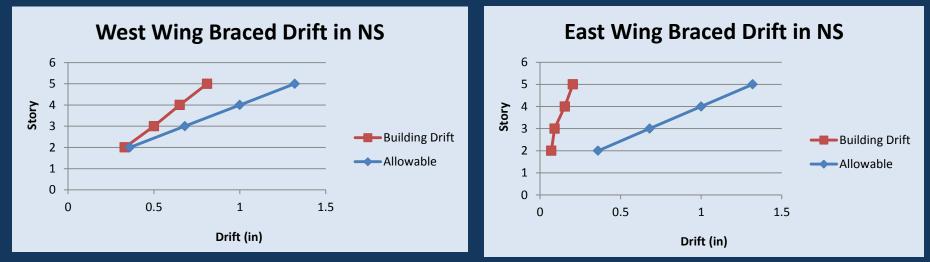
- Building Introduction
- Existing Structural System
- Problem Statement
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- Lateral Frame Designs
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#### BRACED FRAME DESIGN

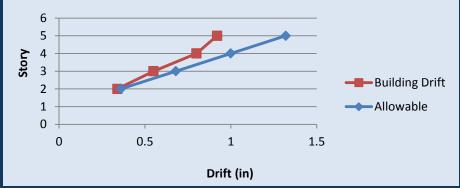
#### Frame A Sizes



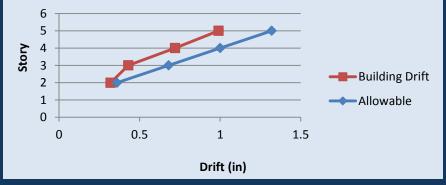
#### Braced Frame Drift



West Wing Braced Drift in EW



#### East Wing Braced Drift in EW



<ul> <li>Building Introduction</li> </ul>		
<ul> <li>Existing Structural System</li> </ul>		
<ul> <li>Problem Statement</li> </ul>	Cost	
<ul> <li>Proposed Solution</li> </ul>	Cost	
<ul> <li>New Lateral Loads</li> </ul>	Percent	
<ul> <li>Lateral Frame Designs</li> </ul>		
<ul> <li>Façade Design Breadth</li> </ul>		
<ul> <li>Acknowledgements</li> </ul>	B	Buildin
		Orig
	Height	9
	Difference	N

#### COMPARISONS

#### Estimated Cost Analysis For Frame A

Original	Moment	Braced
\$ 186,281.00	\$ 567,043.00	\$ 202,572.00
100%	304%	109%

ding Height Change					
Priginal	Moment	Braced			
93'	98'	92'			
N/A	5'	-1'			

<ul> <li>Building Introduction</li> </ul>		
<ul> <li>Existing Structural System</li> </ul>		
<ul> <li>Problem Statement</li> </ul>	Cost	
<ul> <li>Proposed Solution</li> </ul>	Cost	
<ul> <li>New Lateral Loads</li> </ul>	Percent	
<ul> <li>Lateral Frame Designs</li> </ul>		
<ul> <li>Façade Design Breadth</li> </ul>		
<ul> <li>Acknowledgements</li> </ul>	B	Buildin
		Orig
	Height	9
	Difference	N

#### COMPARISONS

Estimated Cost Analysis For Frame A					
Original	Moment	Braced			
\$ 186,281.00	\$ 567,043.00	\$ 202,572.00			
100%	304%	109%			

ling Height Change					
riginal	Moment	Braced			
93'	98'	92'			
N/A	5'	-1'			

	Typical Member Size between 1 <sup>st</sup> and 2 <sup>nd</sup> Floor on							
		Fra	am	ne A				
		Original		Momer	nt	Brace	d	
	Beam in NS	W24x68	)	W36x25	6	W21x6	68	
	Beam in EW	W30x99	)	W40x37	2	W24x7	76	
	Column	W14x257		W14x60	5 W14x1		.76	
	Bracing	N/A		N/A	W14x		90	
		Weight	Со	mparisor				
				Original	N	loment	Bra	ced
Lateral Resisting Members		330 k 12		1220 k	25	6 k		
Т	Total Building Weight		-	18400 k	1	9290 k	186	00 k
	Percentage	е		100%		105%	10	1%

<ul> <li>Building Introduction</li> </ul>		E
<ul> <li>Existing Structural System</li> </ul>		
<ul> <li>Problem Statement</li> </ul>	Cost	ć
<ul> <li>Proposed Solution</li> </ul>	Cost	\$
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<ul> <li>Lateral Frame Designs</li> </ul>		
<ul> <li>Façade Design Breadth</li> </ul>		
<ul> <li>Acknowledgements</li> </ul>	B	Building H
		Origina
	Height	93'
	Difference	N/A

#### COMPARISONS

	Estimated Cost Analysis For Frame A					
	0	riginal	Mom	ent	Braced	
	\$ 18	36,281.00	\$ 567,043.00		\$ 202,572.00	
		100%		304%	109%	
dir	ng Heig	ht Change			Moment Frame	
	ginal 93'	Moment 98'	Braced 92'		have more Architectural Freedom	
Ν	N/A	5'	-1'			

	Typical Member Size between 1 <sup>st</sup> and 2 <sup>nd</sup> Floor on						
		Fra	me A				
		Original	Mome	nt	Brace	d	
	Beam in NS	W24x68	W36x2	56	W21x(	68	
	Beam in EW	W30x99	W40x3	72	W24x	76	
	Column	W14x257	W14x257 W14x605		W14x176		
	Bracing	N/A	N/A		W14x90		
		Weight (	Compariso	n			
			Original	N	loment	Bra	ced
Lateral Resisting Members		/lembers	330 k	-	1220 k	25	6 k
Total Building Weight		Veight	18400 k	1	9290 k	186	00 k
Percentage		e	100%		105%	10	1%

- Building Introduction
- Existing Structural System
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MAT Foundation Design

- Great for Soil with Low Bearing Capacity
- Great for Large Column Loads
- Soil Bearing Capacity of 2500 psf
- Design is Very Complex

#### FOUNDATION DESIGN

Foundation Summary						
Original Moment Braced						
F.S. Bearing	N/A	2.8	2.8			
F.S. Uplift	N/A	Not an issue	4.4			
F.S. Strength	N/A	2.5	2.5			
Depth into Earth	8'-8"	10'	11'-6"			
Thickness of MAT	4'	6'	7'-6"			

<ul> <li>Building Introduction</li> </ul>	ETABS Model
<ul> <li>Existing Structural System</li> </ul>	• AE 530, Co
<ul> <li>Problem Statement</li> </ul>	
<ul> <li>Proposed Solution</li> </ul>	Trutical Ond Ela
<ul> <li>New Lateral Loads</li> </ul>	Typical 2 <sup>nd</sup> Flo
<ul> <li>Lateral Frame Designs</li> </ul>	• AE 534, Ste
<ul> <li>Façade Design Breadth</li> </ul>	
<ul> <li>Acknowledgements</li> </ul>	Façade Breadtl
	• AE 542 Bu

#### MAE REQUIREMENT

## CONNECTIONS

#### 1

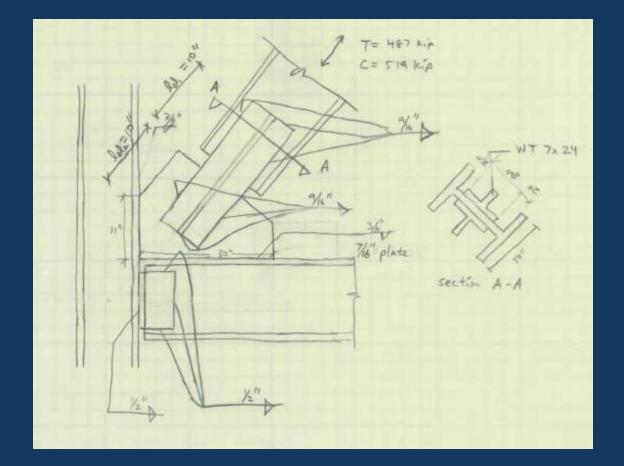
#### omputer Modeling

oor Brace Connection

eel Connections

#### h

uilding Enclosures



<ul> <li>Building Introduction</li> </ul>	Rain Scre
<ul> <li>Existing Structural System</li> </ul>	• Ter
<ul> <li>Problem Statement</li> </ul>	Val
<ul> <li>Proposed Solution</li> </ul>	•
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<ul> <li>Façade Design Breadth</li> </ul>	•
<ul> <li>Acknowledgements</li> </ul>	•

# FAÇADE DESIGN

#### een Wall Cladding System

- rraClad Rain Screen manufactured by Boston lley Terra Cotta
- Simple to Install
- Shield from wind driven rain
- LEED credit opportunities
- Abundant colors and sizes, match original
- 6" additional thickness to exterior wall



### TerraClad RAIN SCREEN

#### Florida Building Code - High Velocity Hurricane Zone Testing, Miami-Dade County NOA08-1014.03

#### TAS 201-94

Impact Test Procedures - Large Missile Impact

#### TAS 202-94

Criteria for Testing Products Subject to Cyclic Wind Pressure Loading

#### TAS 203-94

Criteria for Testing Impact & Non Impact Resistant Building Envelope Components Using Uniform Static Air Pressure

http://www.bostonvalley.com/terraclad/product-testing.html

<ul> <li>Building Introduction</li> </ul>	Rain Scre
<ul> <li>Existing Structural System</li> </ul>	• Ter
<ul> <li>Problem Statement</li> </ul>	Val
<ul> <li>Proposed Solution</li> </ul>	•
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# FAÇADE DESIGN

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The Bechtler Museum of Modern Arts





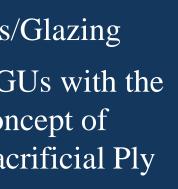


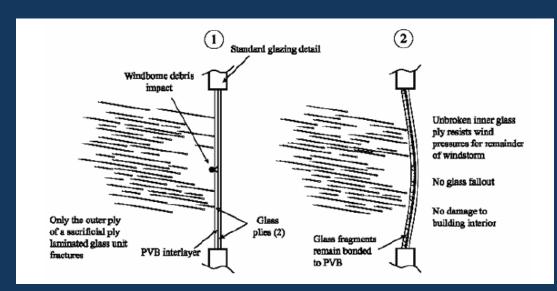
The Colburn School of Performing Arts

<ul> <li>Building Introduction</li> </ul>	Windows
<ul> <li>Existing Structural System</li> </ul>	• LC
<ul> <li>Problem Statement</li> </ul>	COI
<ul> <li>Proposed Solution</li> </ul>	Sa
<ul> <li>New Lateral Loads</li> </ul>	
<ul> <li>Lateral Frame Designs</li> </ul>	
<ul> <li>Façade Design Breadth</li> </ul>	
<ul> <li>Acknowledgements</li> </ul>	
<ul> <li>Acknowledgements</li> </ul>	



# FAÇADE DESIGN





Typical Window Design						
	Width	Height	Outer Ply Thickness	Inner Ply Thickness		
2'x4'	2'	4'	1/8"	3/16"		
5'x10'	6'	10'	1/8"	5/8"		



- Heather Sustersic
- Highland Associates
  - Eric McAndrew
- o TCMC • Family and Friends

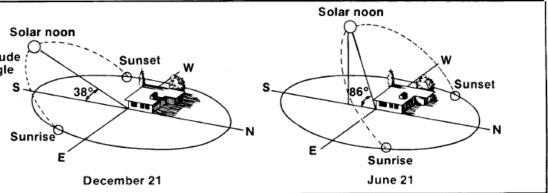
#### ACKNOWLEDGEMENTS

• Penn State Architectural Engineering Faculty



# Appendix

#### SOLAR PANEL DESIGN



http://www.kahnsolar.com/images/how\_solar\_works\_pic.jpg

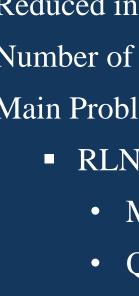
- HIT Power 220A Photovoltaic Module, by Panasonic
  - Withstand 60 psf
  - Top Energy Producer
  - Highest Output on Cloudy Days

Estimated Life-Cycle Cost for 20 years= \$ 279,086 Estimated Total Savings = \$ 10,000 Estimated Payback Period = 27 years



The Bechtler Museum of Modern Arts





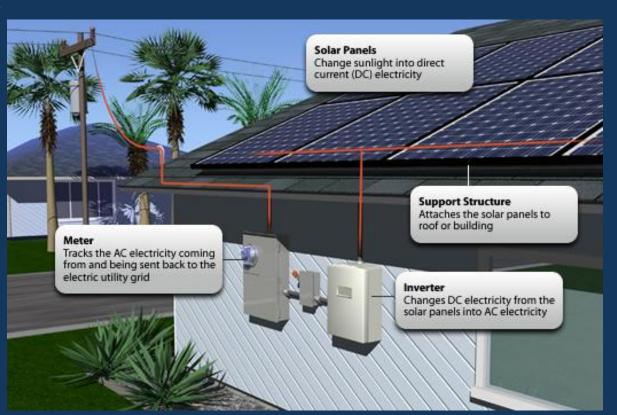
## MECHANICAL AND ELECTRICAL

#### Reduced in Number of Steam Boilers

- Number of McQuay Chillers for Cooling stayed the same
- Main Problem High Humidity
  - RLNL-G Dehumidifier by Rheem
    - Money Saving Efficiency
    - Quiet Operation
    - Quality
    - Remote Monitoring and Control

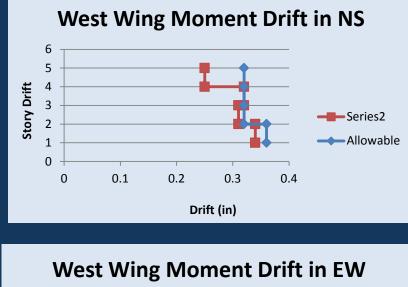
#### Grid-Tied System

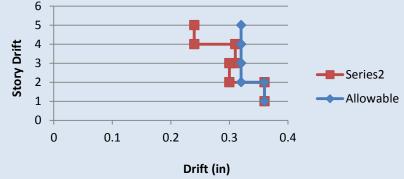
• Net-metering



#### **GRID-TIED CONNECTION**

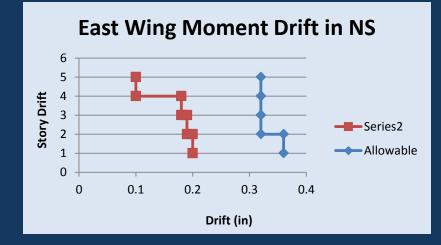
#### Image from Kahn Solar



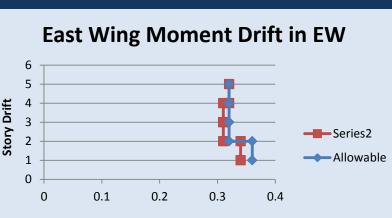


## BUILDING DRIFT

#### Moment Frame Drift

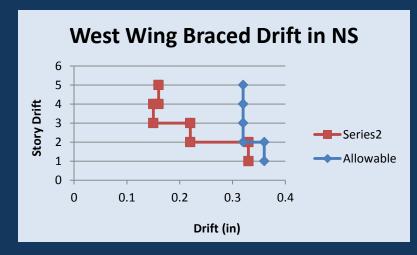




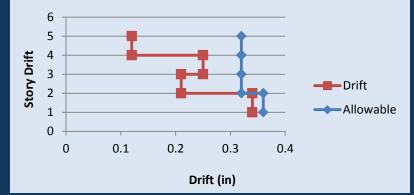


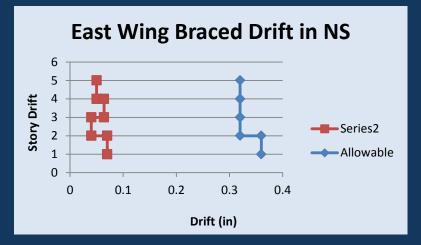
Drift (in)

#### Braced Frame Drift

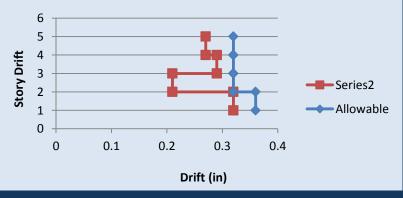


#### West Wing Braced Drift in EW





East Wing Braced Drift in EW



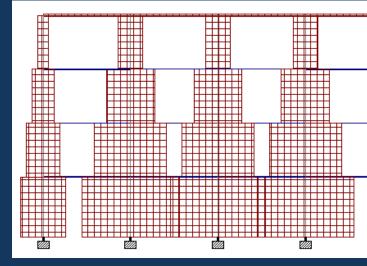


The High-Velocity Hurricane Zones (HVHZ) are specifically defined as Miami-Dade and Broward Counties. As in previous editions of the FBCB, a single wind speed is used for the HVHZ for each Risk Category Map. The design wind speeds in the HVHZ are as follows:

Miami-Dade County Risk Category I Buildings and Structures: 165 mph Risk Category II Buildings and Structures: 175 mph Risk Category III and IV Buildings & Structures: 185 mph

## ASCE 7-10 CODE DIFFERENCE

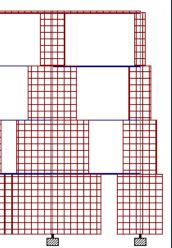
#### High-Velocity Hurricane Zones

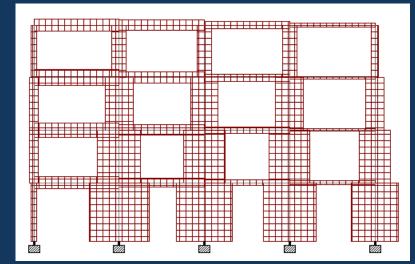


1.2D + 1.6L + 0.5 Lr

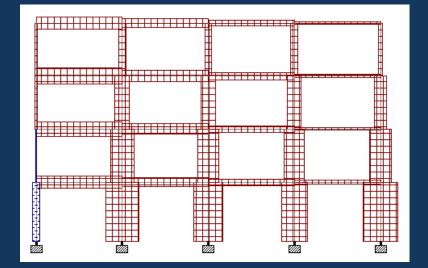
#### MOMENT FRAMES AXIAL

#### Frame A

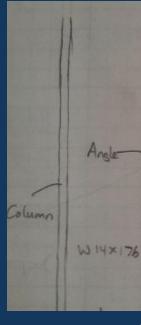




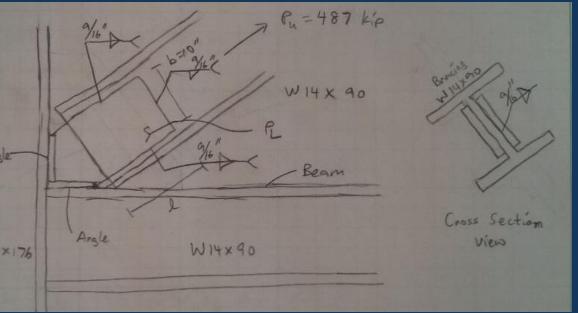
1.2D + 1.6W + L + 0.5Lr



0.9D + 1.6W



### CONNECTIONS



Did not check for Compression Force

