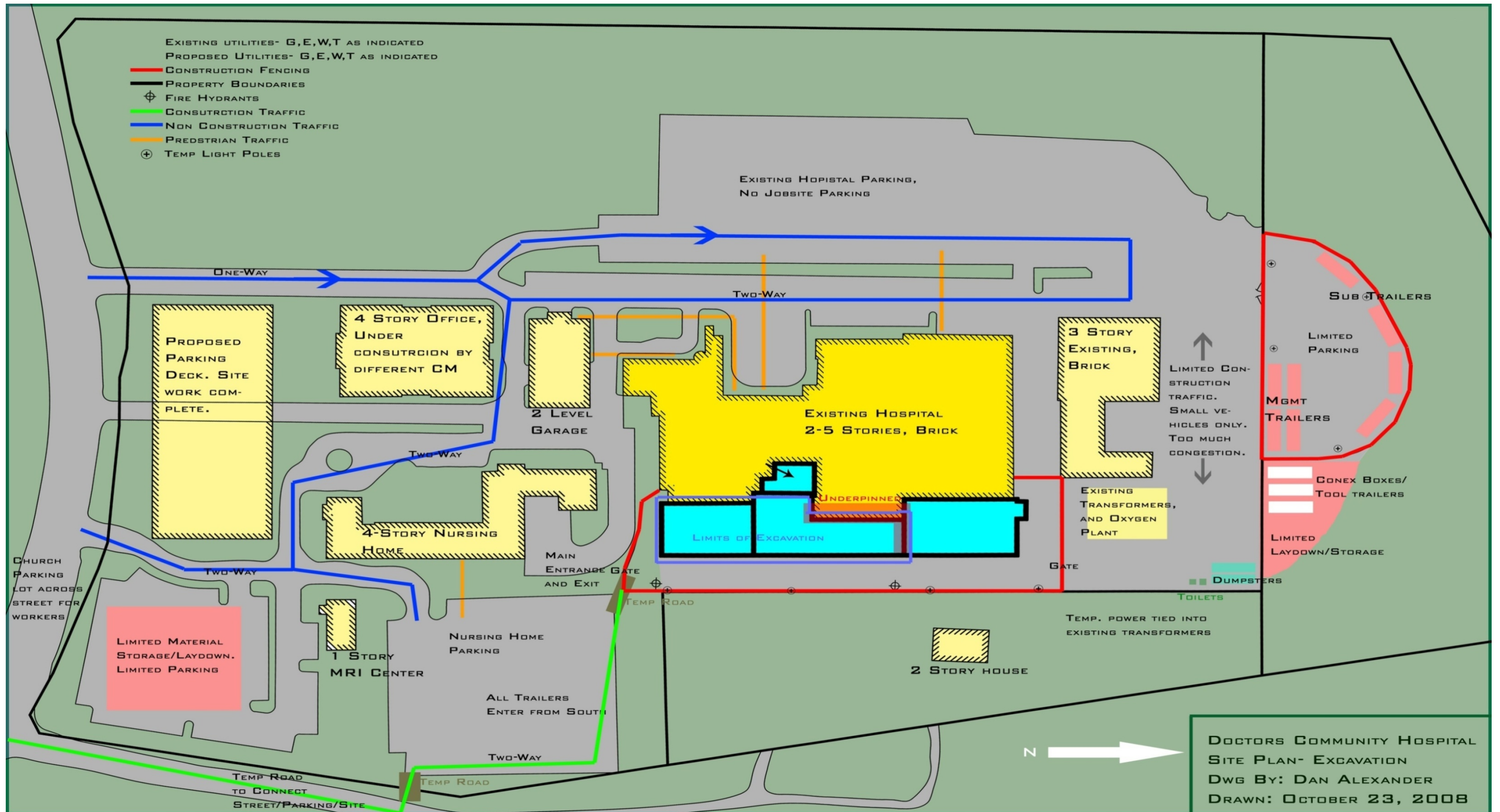
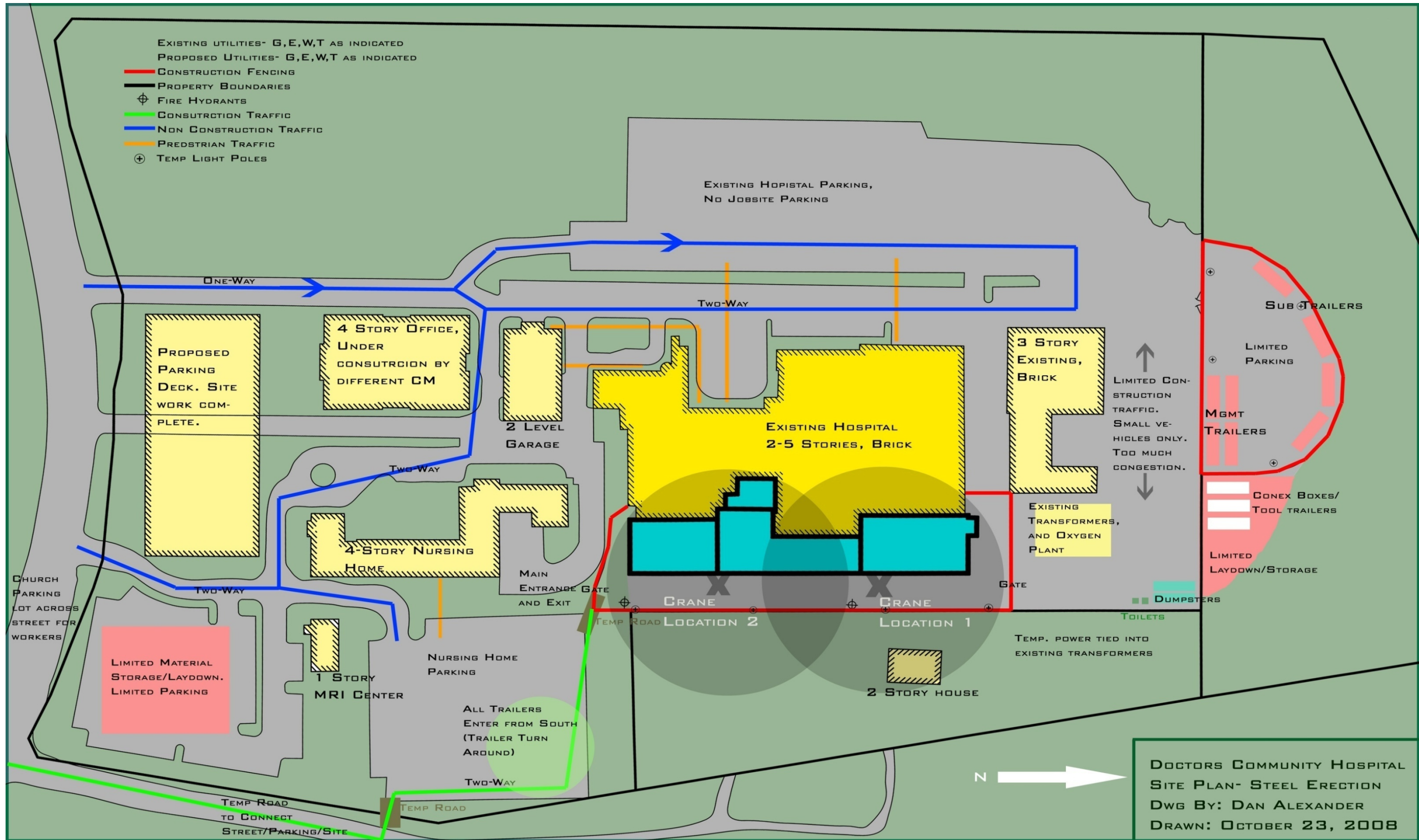
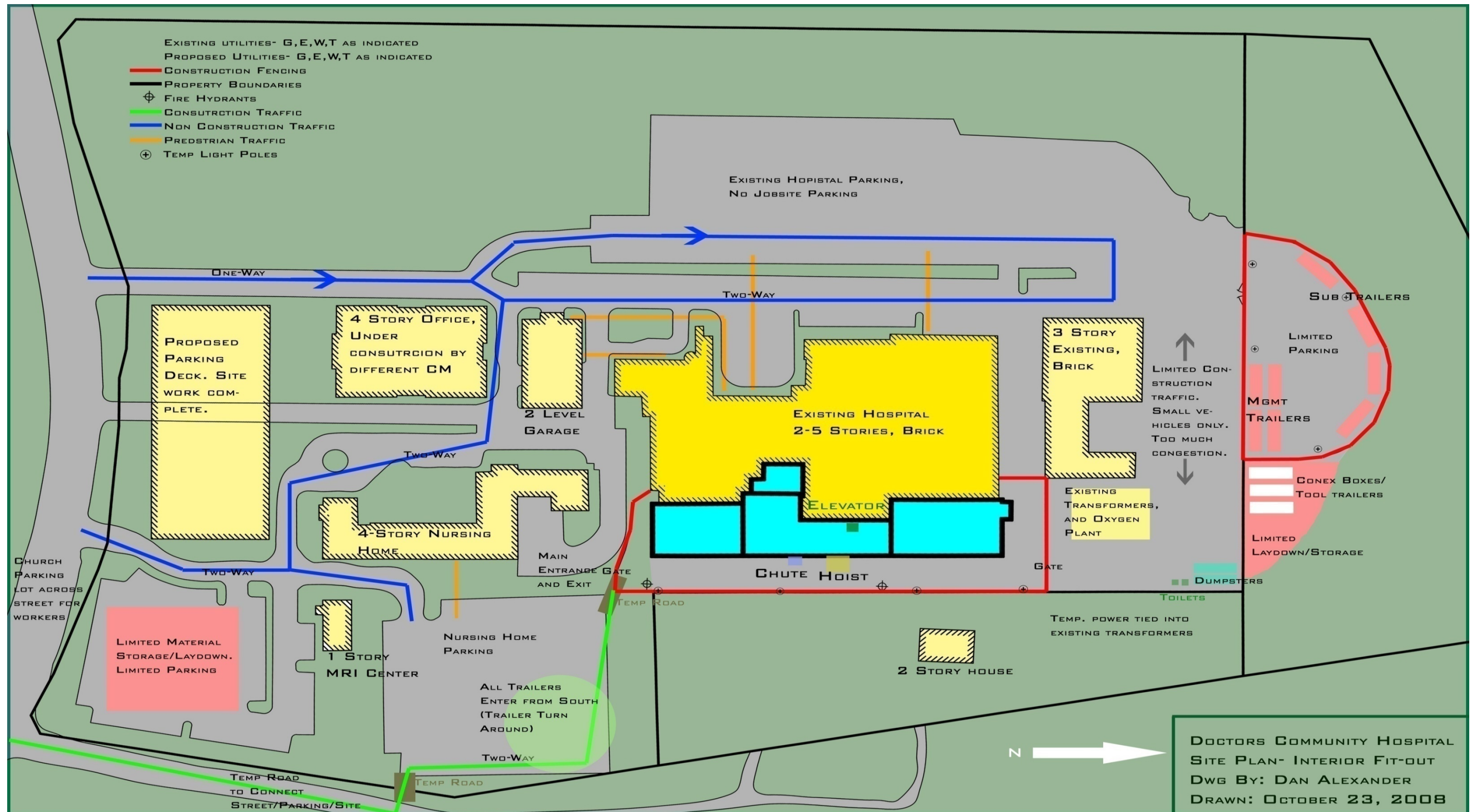


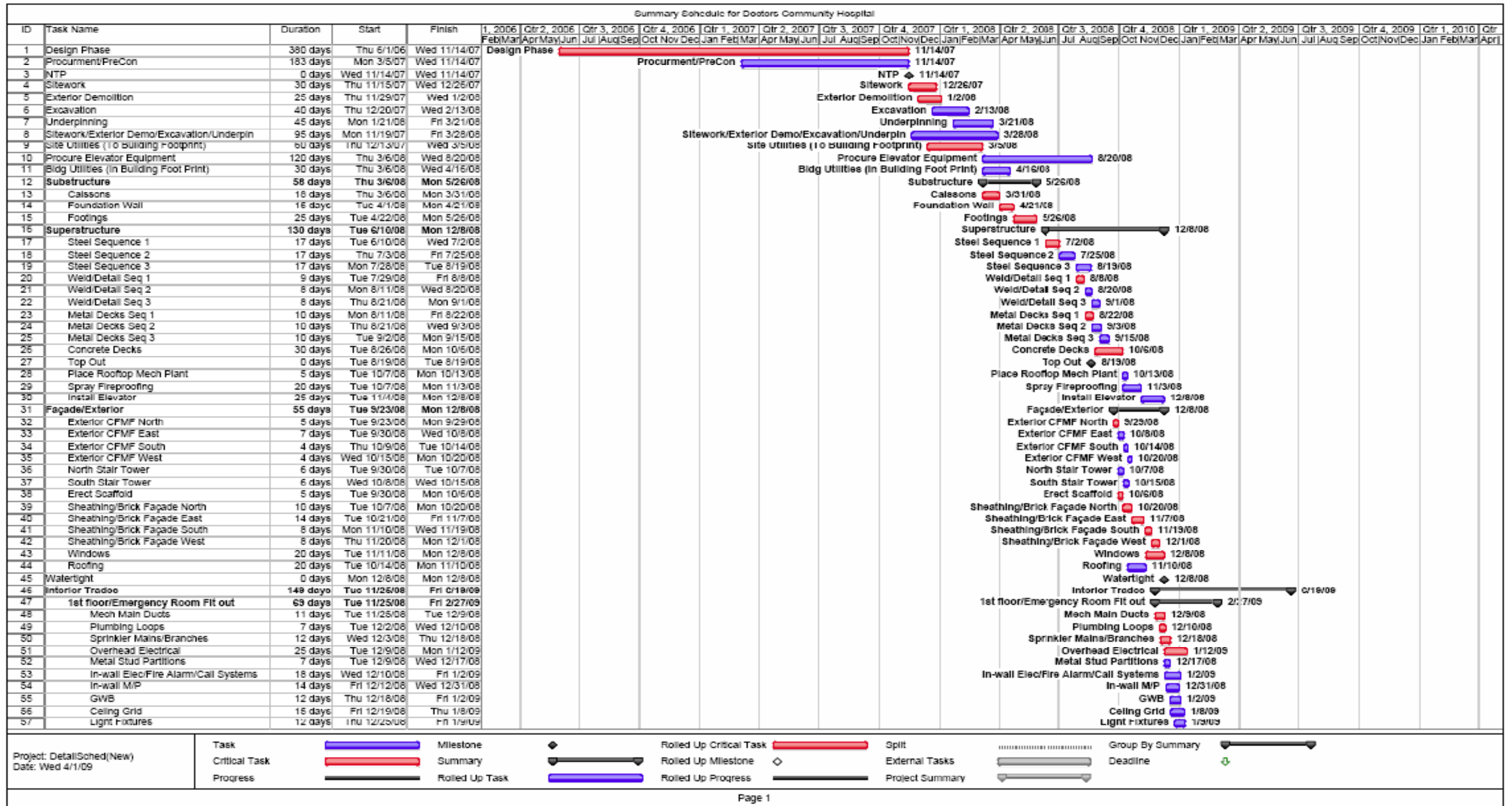
APPENDIX I | SITE LAYOUT PLANS

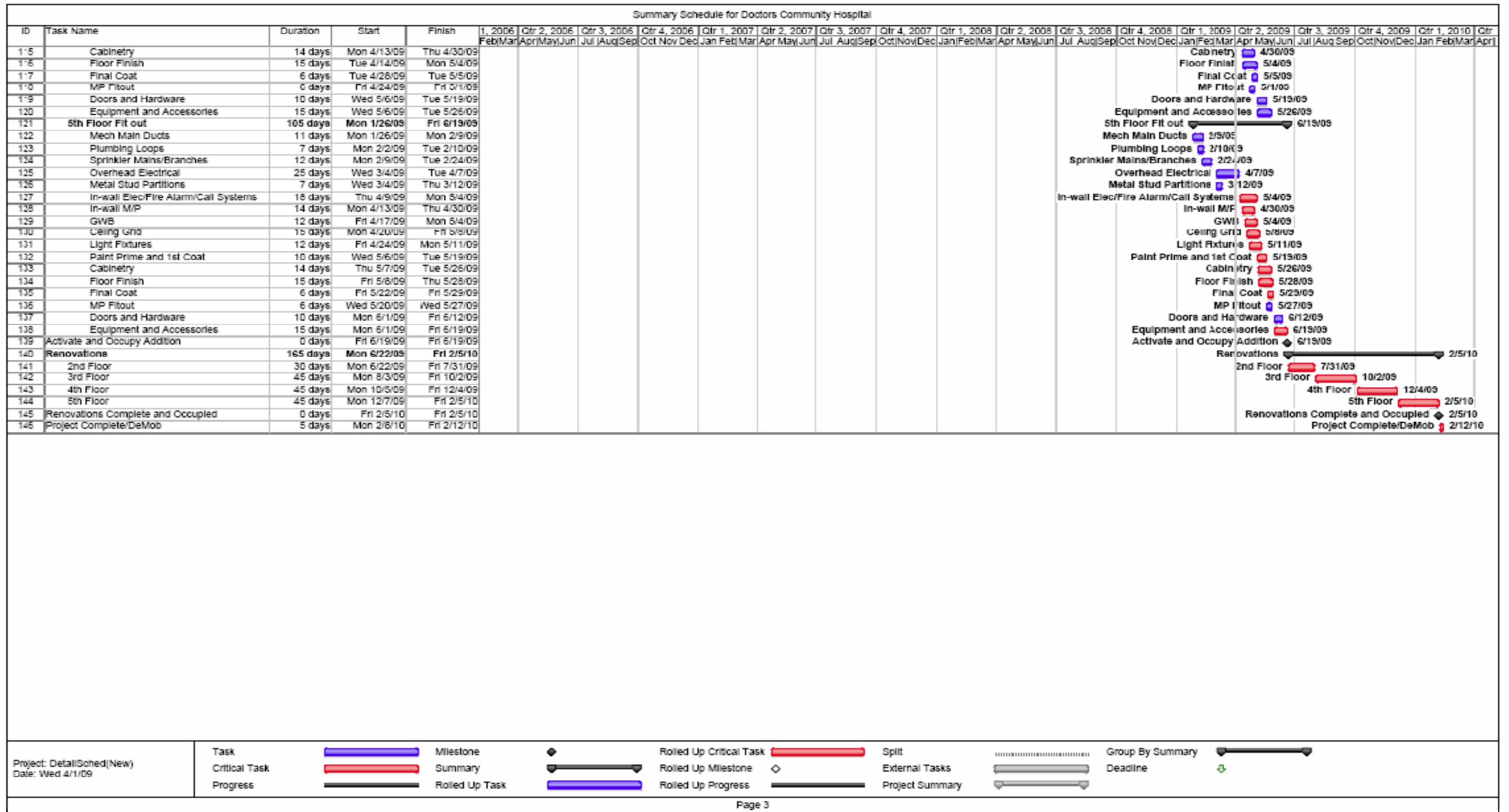






APPENDIX II | CPM SCHEDULE





APPENDIX III | DETAILED ESTIMATE BREAKDOWNS

TABLE 20-DETAILED BREAKDOWN OF GENERAL CONDITIONS ESTIMATE

General Conditions Estimate				
Total Project Weeks		119		
Total Project Months		27		

Personnel	% of time on Project	Total Billable Weeks	Cost per Week	Total Cost
Project Executive	50%	59.5	\$ 2,100	\$ 124,950
Project Manager	100%	119	\$ 1,850	\$ 220,150
Assistant Project Manager	100%	119	\$ 1,600	\$ 190,400
Field Engineer	100%	119	\$ 1,125	\$ 133,875
General Superintendent	70%	83.3	\$ 1,800	\$ 149,940
Assistant Superintendent	100%	119	\$ 1,600	\$ 190,400
Office Manager	100%	119	\$ 800	\$ 95,200
Category Total				\$ 1,104,915

Utilities/Facilities	Frequency	Duration	Cost/Unit Time	Total Cost
Electric/Water	Monthly	27	500	\$ 13,500
Internet	Monthly	27	\$ 300	\$ 8,100
Porta Johns	Weekly	119	\$ 60	\$ 7,140
Telephone	Monthly	27	\$ 600	\$ 16,200
Trailer Set up	Lump Sum	-	-	\$ 10,000
Trailers	Monthly	27	\$ 750	\$ 20,250
Utilities Hook Up	Lump Sum	-	-	\$ 15,000
Category Total				\$ 90,190

Site Office Support	Frequency	Duration	Cost/Unit Time	Total Cost
Cell phone and Nextel	Monthly	27	\$ 300	\$ 8,100
Computers	Lump Sum	-	-	\$ 10,000
Janitorial service for trailer	Monthly	27	\$ 200	\$ 5,400
Job Travel	Monthly	27	\$ 250	\$ 6,750
Job vehicle fuel/maintenance	Monthly	27	\$ 400	\$ 10,800
Job Vehicle/Auto Allowance	Monthly	27	\$ 1,000	\$ 27,000
Office Furniture	Lump Sum	-	-	\$ 5,000
Office Supplies	Monthly	27	\$ 400	\$ 10,800
Postage and Shipping	Monthly	27	\$ 300	\$ 8,100
Category Total				\$ 91,950

General Conditions Estimate (Cont)

General Requirements	Frequency	Duration	Cost/Unit Time	Total Cost
Bid Set Repro Costs/Distribution	Lump Sum	-	- \$	25,000
Copiers and Supplies	Monthly	27	\$ 600	\$ 16,200
Dumpsters	Weekly	119	\$ 650	\$ 77,350
Final Clean	Lump Sum	-	- \$	20,000
Material Hoist	Weekly	21	\$ 1,780	\$ 37,380
Mock-up (Patient Room)	Lump Sum	-	- \$	45,000
Safety and First Aid	Monthly	27	\$ 1,200	\$ 32,400
Signage	Lump Sum	-	- \$	10,000
Snow Removal	Lump Sum	-	- \$	25,000
Survey and Layout	Lump Sum	-	- \$	35,000
Temp Fence	Monthly	27	\$ 550	\$ 14,850
Temp Ladders/Stairs/Ramps	Lump Sum	-	- \$	30,000
Temp Roads	Lump Sum	-	- \$	50,000
Trash Chute	Weekly	22	\$ 550	\$ 12,100
Category Total				\$ 430,280
General Conditions Total				\$ 1,717,335

TABLE 21-DETAILED STRUCTURAL ESTIMATE

Detailed Structural Estimate								
Steel								
	Quantity	Unit	Material	Labor	Equipment	Total Unit Cost	Total	
Columns								
HSS6X6X5/16	13	EA	\$ 297.00	\$ 43.50	\$ 29.00	\$ 369.50	\$ 4,803.50	
W10X49	39	LF	\$ 54.50	\$ 2.27	\$ 1.52	\$ 58.29	\$ 2,273.31	
W12X106	52	LF	\$ 140.00	\$ 2.55	\$ 1.68	\$ 144.23	\$ 7,499.96	
W12X136	84	LF	\$ 150.00	\$ 2.55	\$ 1.68	\$ 154.23	\$ 12,955.32	
W12X170	68	LF	\$ 230.00	\$ 2.57	\$ 1.72	\$ 234.29	\$ 15,931.72	
W12X40	135	LF	\$ 57.00	\$ 2.27	\$ 1.52	\$ 60.79	\$ 8,206.65	
W12X53	239	LF	\$ 63.00	\$ 2.27	\$ 1.52	\$ 66.79	\$ 15,962.81	
W12X58	26	LF	\$ 68.00	\$ 2.30	\$ 1.52	\$ 71.82	\$ 1,867.32	
W12X65	660	LF	\$ 77.00	\$ 2.32	\$ 1.54	\$ 80.86	\$ 53,367.60	
W12X72	68	LF	\$ 84.00	\$ 2.35	\$ 1.56	\$ 87.91	\$ 5,977.88	
W12X79	106	LF	\$ 93.00	\$ 2.35	\$ 1.57	\$ 96.92	\$ 10,273.52	
W12X87	262	LF	\$ 105.00	\$ 2.38	\$ 1.59	\$ 108.97	\$ 28,550.14	
W8X31	1480	LF	\$ 37.50	\$ 2.17	\$ 1.45	\$ 41.12	\$ 60,857.60	
W8X35	226	LF	\$ 42.00	\$ 2.19	\$ 1.47	\$ 45.66	\$ 10,319.16	
W8X40	216	LF	\$ 49.00	\$ 2.24	\$ 1.49	\$ 52.73	\$ 11,389.68	
W8X48	169	LF	\$ 58.00	\$ 2.27	\$ 1.52	\$ 61.79	\$ 10,442.51	
W8X58	93	LF	\$ 68.00	\$ 2.32	\$ 1.55	\$ 71.87	\$ 6,683.91	
W8X67	282	LF	\$ 81.00	\$ 2.38	\$ 1.59	\$ 84.97	\$ 23,961.54	
Beams								
W10X12	335.07	LF	\$ 14.50	\$ 3.91	\$ 2.61	\$ 21.02	\$ 7,043.17	
W12X14	718.6	LF	\$ 16.95	\$ 2.66	\$ 1.78	\$ 21.39	\$ 15,370.85	
W12X19	2361.84	LF	\$ 24.00	\$ 2.66	\$ 1.87	\$ 28.53	\$ 67,383.30	
W12X22	159.1	LF	\$ 26.50	\$ 2.66	\$ 1.87	\$ 31.03	\$ 4,936.87	
W12X30	180.22	LF	\$ 35.00	\$ 2.76	\$ 1.90	\$ 39.66	\$ 7,147.53	
W12X35	709.25	LF	\$ 42.50	\$ 2.89	\$ 1.93	\$ 47.32	\$ 33,561.71	
W12X40	280.05	LF	\$ 48.00	\$ 2.93	\$ 1.97	\$ 52.90	\$ 14,814.65	
W14X22	6816.6	LF	\$ 28.50	\$ 2.35	\$ 1.55	\$ 32.40	\$ 220,857.84	
W14X26	126.82	LF	\$ 31.50	\$ 2.37	\$ 1.58	\$ 35.45	\$ 4,495.77	
W16X26	2097.62	LF	\$ 31.50	\$ 2.37	\$ 1.58	\$ 35.45	\$ 74,360.63	
W16X31	97.76	LF	\$ 37.50	\$ 2.60	\$ 1.74	\$ 41.84	\$ 4,090.28	
W16X36	1273.93	LF	\$ 44.50	\$ 2.87	\$ 1.90	\$ 49.27	\$ 62,766.53	
W16X40	516.18	LF	\$ 48.50	\$ 2.93	\$ 1.96	\$ 53.39	\$ 27,558.85	
W18X35	44.76	LF	\$ 42.50	\$ 3.53	\$ 1.77	\$ 47.80	\$ 2,139.53	
W18X40	130.67	LF	\$ 48.50	\$ 3.53	\$ 1.77	\$ 53.80	\$ 7,030.05	
W18X50	195	LF	\$ 60.50	\$ 3.72	\$ 1.86	\$ 66.08	\$ 12,885.60	
W21X44	52	LF	\$ 53.00	\$ 3.19	\$ 1.60	\$ 57.79	\$ 3,005.08	
W21X50	26	LF	\$ 60.50	\$ 3.19	\$ 1.60	\$ 65.29	\$ 1,697.54	
W21X57	168	LF	\$ 69.00	\$ 3.24	\$ 1.62	\$ 73.86	\$ 12,408.48	
W21X68	281.5	LF	\$ 82.50	\$ 3.27	\$ 1.64	\$ 87.41	\$ 24,605.92	
W24X68	56	LF	\$ 82.50	\$ 3.06	\$ 1.53	\$ 87.09	\$ 4,877.04	
W24X76	55.5	LF	\$ 92.00	\$ 3.06	\$ 1.53	\$ 96.59	\$ 5,360.75	
W24X94	29.5	LF	\$ 114.00	\$ 3.14	\$ 1.57	\$ 118.71	\$ 3,501.95	
W8X15	34.68	LF	\$ 18.15	\$ 3.81	\$ 2.61	\$ 24.57	\$ 852.09	
W8X18	15	LF	\$ 21.00	\$ 3.84	\$ 2.63	\$ 27.47	\$ 412.05	
Metal Deck								
1 1/2" 18 Gauge	67861	SF	\$ 1.36	\$ 0.32	\$ 0.03	\$ 1.71	\$ 116,042.31	
						Steel Total	\$ 1,030,530.47	

Detailed Structural Estimate (Cont)							
Concrete							
	Quantity	Unit	Material	Labor	Equipment	Total Unit Cost	Total
Foundations							
Spread Footings (1-5 CY)	367	CY	\$ 192.00	\$ 95.50	\$ 0.57	\$ 288.07	\$ 105,721.69
Caissons	550	VLF	\$ 56.50	\$ 57.50	\$ 66.00	\$ 180.00	\$ 99,000.00
Grade Wall	10	CY	\$ 228.00	\$ 279.00	\$ 27.50	\$ 534.50	\$ 5,345.00
Floors							
Slab on Grade (6")	17423	SF	\$ 1.95	\$ 0.75	\$ 0.01	\$ 2.71	\$ 47,216.33
Concrete on Metal Deck (6")	67861	SF	\$ 2.02	\$ 0.73	\$ 0.28	\$ 3.03	\$ 205,618.83
6x6 WWF Reinforcing	852.84	CSF	\$ 29.00	\$ 25.50	\$ -	\$ 54.50	\$ 46,479.78
					Concrete Total		\$ 509,381.63

STRUCTURAL TOTAL:	\$	1,539,912.10
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APPENDIX IV | PROCESS MODEL

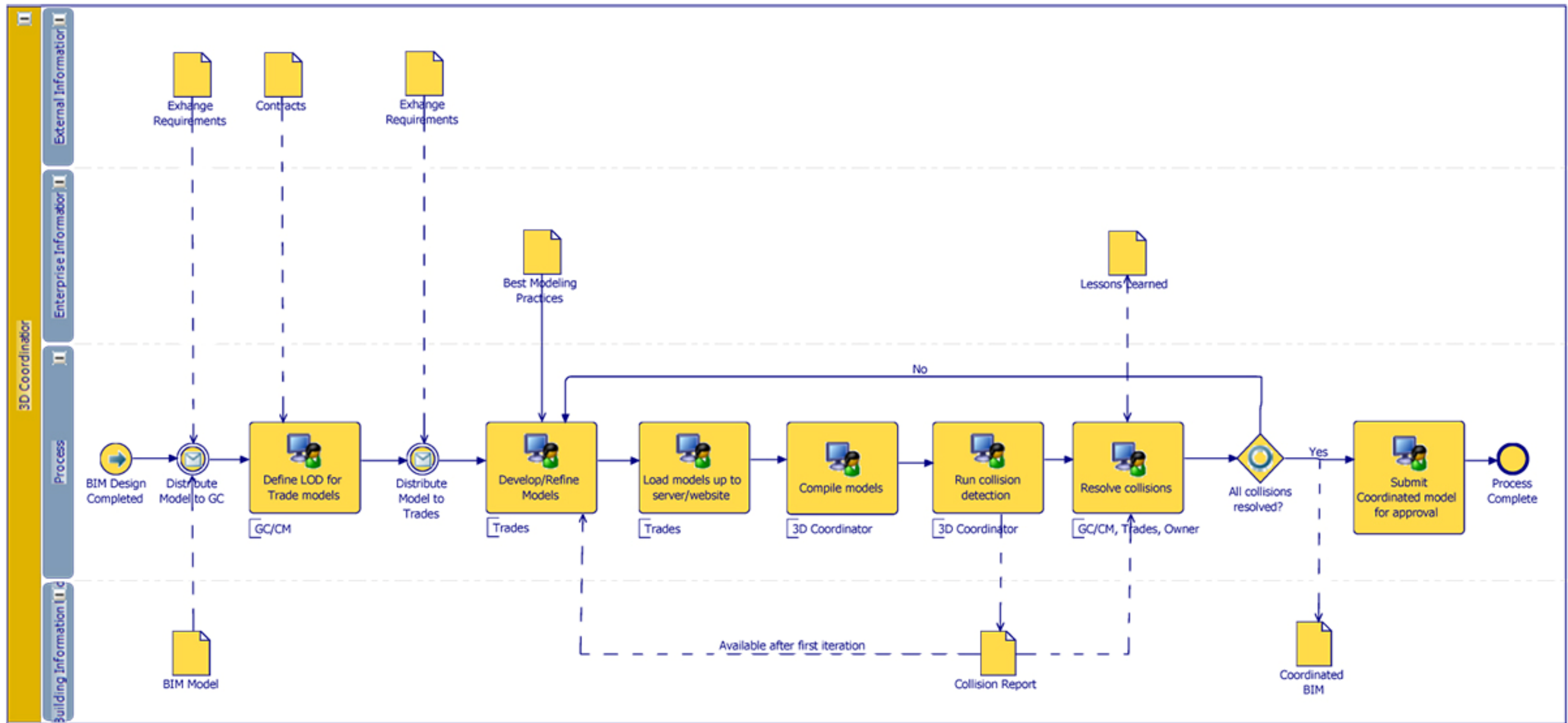


TABLE 22-EXPLANATION OF TASKS AS DEFINED IN PROCESS MODEL

Task Name	Explanation of Task and Related Data Objects
<p>Define LOD for Trade Models</p>	<p>A level of detail must be defined in order for trades to accurately model the systems in order for 3D coordination to be effective. This stage will define what must be modeled. Some items that are typical question marks on whether to be included are:</p> <ul style="list-style-type: none"> • Hangers, pipe supports, sleeves? • Conduits? • Pipe/Duct insulation? • Metal deck detail? <p>According to Leicht and Messner, four main factors weigh into the determination for the level of detail:</p> <ul style="list-style-type: none"> • Interaction with other systems • Sequence of Installation • Prefabrication Components • Layout considerations and density of systems <p><u>Contracts</u>- External information that will impact the contract language in the trades agreement (risk allocation, intellectual property licensure, etc.) and definitions for the LOD necessary for each trade.</p>
<p>Develop/Refine Models</p>	<p>This task consists of the actual work done to create the model. Time will be spent here by the trades or their consultants actually developing the 3D models to be used for coordination. Typically, this will consist of developing the model for one area of the building at a time.</p> <p><u>Best Modeling Practices</u>- This data object represents enterprise information in the form of lessons learned and best ways to represent information in the 3D model. It will impact how trades/consultants will model the necessary information.</p>
<p>Load Models up to server/website</p>	<p>An FTP server or website should be maintained by the coordination leader in order to facilitate the transfer of the model files which can become quite large. Typically, e-mail will not have sufficient space for these files to be sent as attachments. Each trade will be responsible to upload their model for a given area by a specific date as determined by the coordinator. Files should be uploaded in a compatible format with the software that will be used for collision detection.</p>

Compile Models	The leader for the 3D coordination will assemble the models into one file/file set in order to run the collision detection.
Run Collision Detection	The 3D coordinator will run the collision detection to find all conflicts between the models. At this point, the 3D coordinator can remove false positives depending on LODs that were previously determined. At the conclusion of this activity, a collision report will be outputted and distributed to the trades.
Resolve Collisions	Decisions will be made by the necessary participants to resolve each clash. Coordination issues will be resolved based on trade inputs. Design issues will result in RFI's. Clashes resulting from LOD (Leicht and Messner, 2008) The steps to resolve the collisions will be determined on a project or company level.
Submit Coordinated Model for Approval	The coordinated model is submitted back to designers for final approval.

TABLE 23-EXPLANATION OF EVENTS AS DEFINED IN PROCESS MODEL

Event Name	Explanation of Event and Related Data Objects
BIM Design Complete	This is the start to the 3D coordination process. The designers have completed the overall design intent for the project.
Distribute Model to GC	A transfer based event, in which the model is sent to the GC or CM on the project. <u>Exchange Requirements</u> - These must be defined by the project team and will determine what file formats will be used on the project to complete the 3D coordination. This is information that can be defined from an external resource that is not taken from either the model or internal enterprise information. <u>BIM Model</u> -Data taken from the BIM model (in this case the model itself) is an information input for this task.
Distribute Model to Trades	Another transfer based event, in which the model is sent to the trades in order for them to begin their work with actually creating the model for their specific trade. Trades to be included will be defined at the project specific level. <u>Exchange Requirements</u> - Requirements for transfer will be determined in order to define the necessary file formats to be distributed to the trades, and also the formats that they will return to the coordination leader.

APPENDIX V | TAKE-OFF DATA

TABLE 24-REVIT TAKE OFF OF EXTERIOR WALL AREA

Family	Family and Type	Area	Unit
Basic Wall	Basic Wall: For SF Take-Off	1064	SF
Basic Wall	Basic Wall: For SF Take-Off	205	SF
Basic Wall	Basic Wall: For SF Take-Off	9072	SF
Basic Wall	Basic Wall: For SF Take-Off	3817	SF
Basic Wall	Basic Wall: For SF Take-Off	3807	SF
Basic Wall	Basic Wall: For SF Take-Off	4680	SF
Basic Wall	Basic Wall: For SF Take-Off	1725	SF
Basic Wall	Basic Wall: For SF Take-Off	560	SF
Basic Wall	Basic Wall: For SF Take-Off	1040	SF
Basic Wall	Basic Wall: For SF Take-Off	5207	SF
Basic Wall	Basic Wall: For SF Take-Off	600	SF
Basic Wall	Basic Wall: For SF Take-Off	2335	SF
Basic Wall	Basic Wall: For SF Take-Off	1015	SF
Basic Wall	Basic Wall: For SF Take-Off	1015	SF
Basic Wall	Basic Wall: For SF Take-Off	420	SF
Basic Wall	Basic Wall: For SF Take-Off	1315	SF
	Total	37877	SF
	Non Precast Façade Area	750	SF
	Net Total Precast Area	37127	SF

APPENDIX VI | DETAILED STRUCTURAL CALCULATIONS

Exterior Beam:

Live Load Reduction:

$$LL_r = LL \left(.25 + \frac{15}{\sqrt{K_{LL} * A_t}} \right)$$

$$LL_r = 100 \text{ psf} \left(.25 + \frac{15}{\sqrt{2 * 288 \text{ sf}}} \right)$$

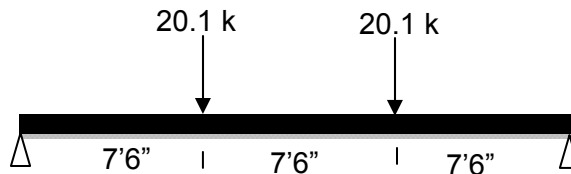
$$LL_r = 100 \text{ psf} (.875) \quad (.875 > .4 \therefore OK)$$

$$LL_r = \mathbf{87.5 \text{ psf}}$$

Beam Shear and Moment Calculations:

$$1.2D + 1.6L = 1.2(58) + 1.6(87.5) = 209.6 \text{ psf}$$

$$209.6 \text{ psf} * 7'6" * 12'10" = 20.1 \text{ kips as point loads on Edge beam}$$



Support Reactions = 20.1 k by symmetry inspection

$$\therefore V_{max} = 20.1 \text{ k}$$

$M_u = V_{max} * Spacing$ (for simply supported beam and point loads)

$$M_u = (20.16 \text{ k}) * 7'6"$$

$$M_u = \mathbf{151.2 \text{ kip ft}}$$

Load due to Exterior Brick Façade:

Brick weight: 42 psf Story Height: 13'

*Distributed load = DL Safety factor * Sq. Ft. Unit Weight * Story Height*

$$Distributed \text{ load} = 1.2 * 42 \text{ psf} * 13'$$

$$Distributed \text{ load} = .655 \text{ klf}$$

For simply supported beam with distributed load :

$$M_{max} = \frac{(Dist. Load * Beam Length^2)}{8}$$

$$M_{max} = \frac{(.65 * 22.5^2)}{8}$$

$$\mathbf{M_{max} = 41.2 kip ft}$$

Exterior Column:

$$KL = 1 * 13 = 13 \text{ for column sizing from AISC steel manual}$$

Live Load Reduction for Calc 1:

$$LL_r = LL \left(.25 + \frac{15}{\sqrt{K_{LL} * A_t}} \right)$$

$$LL_r = 100 \left(.25 + \frac{15}{\sqrt{4 * (288 * 3)}} \right)$$

$$LL_r = 100(.5) .5 > .4 \therefore OK$$

$$\mathbf{LL_r = 50 psf}$$

Axial Loading Calculations for W8x35:

$$1.2D + 1.6L = 1.2(58) + 1.6(50) = 149.6 \text{ psf}$$

$$\text{Axial Load} = \text{Tributary Area} * \text{Load per sq. ft.}$$

$$\text{Axial Load} = \left(288 \frac{\text{sf}}{\text{floor}} * 3 \text{ floors} \right) * 149.6 \text{ psf}$$

$$\mathbf{\text{Axial Load} = 129.2 \text{ kips (excluding facade)}}$$

$$\text{Axial Load}_{\text{facade}} = \left(292.5 \frac{\text{sf}}{\text{story}} * 3 \text{ stories} \right) * 65 \text{ psf}$$

$$\mathbf{\text{Axial Load}_{\text{facade}} = 57 \text{ kips}}$$

$$\mathbf{\text{Total Axial Load} = P_u = 186.2 \text{ kips}}$$

$$\mathbf{\Phi_c P_n = 300 \text{ kips} > 186.2 \text{ kips} = P_u}$$

Live Load Reduction for Calc 2:

$$LL_r = LL \left(.25 + \frac{15}{\sqrt{K_{LL} * A_t}} \right)$$

$$LL_r = 100 \left(.25 + \frac{15}{\sqrt{4 * (288 * 5)}} \right)$$

$$LL_r = 100(.44) \quad .44 > .4 \quad \therefore OK$$

$$LL_r = 44 \text{ psf}$$

Axial Loading Calculations for W8x58:

$$1.2D + 1.6L = 1.2(58) + 1.6(44) = 140 \text{ psf}$$

*Axial Load = Tributary Area * Load per sq. ft.*

$$Axial Load = \left(288 \frac{\text{sf}}{\text{floor}} * 5 \text{ floors} \right) * 140 \text{ psf}$$

Axial Load = 201.6 kips (excluding facade)

$$Axial Load_{facade} = \left(292.5 \frac{\text{sf}}{\text{story}} * 5 \text{ stories} \right) * 65 \text{ psf}$$

$$Axial Load_{facade} = 95.1 \text{ kips}$$

Total Axial Load = $P_u = 296.7$ kips

$$\Phi_c P_n = 514 \text{ kips} > 296.7 \text{ kips} = P_u$$