

The Commonwealth Medical College Scranton, PA



Revised Proposal

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Structural Option

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Executive Summary

The Commonwealth Medical College, TCMC, is a four story building located in Scranton, PA. It was completed in April 2011. The structure of TCMC is composed of a composite steel system and utilizes moment frames for the lateral system.

TCMC was designed for the expected wind, seismic and weather loads in Scranton area. Therefore, if an unexpectedly powerful hurricane were to hit the Scranton area, TCMC may not be able to resist it. Because of this, this structural depth proposal is to investigate the increase in wind and weather loads in a design process by theoretically relocating TCMC to Miami, FL, a hurricane prone region. Higher design wind loads, from hurricane winds, and impact from debris will be considered.

The codes that will be used to design TCMC in Miami are the Florida Building Code 2010, and ASCE 7-05. Two main systems will be designed and compared in this proposal; moment frames and laterally braced frames. Moment frames are the original system for TCMC. It will be designed to withstand a larger wind load since it is now in Miami. This frame will be compared to a lateral braced frame system, which is the second system proposed. Both systems will stay with the original gravity system of structural steel. A new foundation will be designed to account for the different soil condition in Miami Florida. Also, the foundations for the two proposed system will be different due to the different lateral systems. The intent of the new designs is to show the difference in the amount of structural impact and cost between the new one and the original one.

In addition to the structural depth proposal, three breadths are also proposed. The first breadth is on façade design, concentrating on heat transfer, waterproofing, and day lighting given the new site, along with impact load consideration from the structural depth. Miami, FL has more moisture in the air and more day lighting compared to Scranton, PA.

The second breath is on solar panel design. It is easy to see the great opportunities for solar energy in Florida, so a new photovoltaic system will be very beneficial. TCMC can use the readily available abundant amounts of solar energy and convert it to electricity that can be used. This will make TCMC more energy efficient and will potentially increase its LEED rating.

And the last breath is on mechanical and electrical changes. The weather conditions in Miami are very different compared to Scranton. Because of this, the mechanical systems need to be analyzed. Most heating units will be replaced by cooling units for the warmer climate. The addition of solar panels will impact electrical wiring. The electrical system will be wired so that it can use electricity more efficiently. Lastly, a more powerful emergency backup power will be needed in case of a powerful hurricane storm.

MAE coursework will be incorporated into the new design of TCMC. This includes building modeling techniques and many other graduate courses. Tasks and schedule are set forth in this proposal and will be presented in later sections.

Building Introduction

The Commonwealth Medical College (TCMC), also known as The Medical Sciences Building (MSB), is a medical school located in the heart of Scranton, PA. Costing over \$120 million, this four story building, with an additional penthouse on the roof, was completed in April, 2011. The architecture was intended to complement the existing schools and hospitals in the surrounding area. Shown in Figure 1 is the building footprint of TCMC, highlighted in yellow, and the surrounding site.

TCMC is clad in brick, stone, and glass curtain wall. The building is separated into two individual wings, west wing and east wing. The link is the lobby area that connects the two wings and it is clad largely in insulated glass units to let natural sunlight in. An additional feature is the tower which is also clad largely in glass, as shown in Figure 2. The tower, located in the East wing, is considered the main focal point of the building. The interior space of the tower is mainly corridors and small meeting rooms so the students can enjoy the view.

TCMC is a multi-use building, using all modern technology. It has a library where students go for information, Clinical Skills and Simulation Center where students learn from beyond classrooms, lecture halls that can seat up to 160 students, classrooms with Wi-Fi connections, small group meeting rooms where a team of students can work together, and a luxurious student lounge for study or relaxation. Figure 3 shows the interior lobby of TCMC. TCMC also has a garden around the link that allows the occupants to enjoy the nice green views that the city cannot offer. The building is 93 feet tall, 185,000 square feet of space, and is a composite steel framed building that utilizes moment frames for its lateral system.

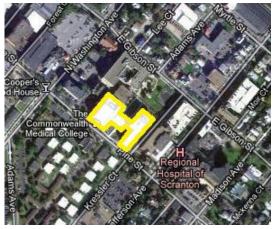


Figure 1 Aerial map from Google.com showing the location of the building site



Figure 2 Picture of the exterior showing the glass and brick facade on the TCMC. The Tower is shown, made will all glass walls. http://www.hok.com



Figure 3 Interior picture of the TCMC lobby. http://www.hok.com

Structural Overview

Design Codes

According to Sheet LS100, the building was designed to comply with:

*	Building Code	2006 International Building Code (IBC)
*	Mechanical	2006 International Mechanical Code
*	Electrical	2005 NFPA 70/ Nation Electrical Code
*	Plumbing	2006 International Plumbing Code
		2006 International Fuel Gas Code
**	Fire Protection	2006 International fire Code

All concrete work conforms to the requirements of the American Concrete Institute ACI-318-05.

Additional Code Reference from American Concrete Institute:

- **❖** ACI-211
- **❖** ACI-301
- **❖** ACI-302
- **❖** ACI-304
- **❖** ACI-305
- **❖** ACI-306
- **❖** ACI-315
- **❖** ACI-347

Regulatory Guidelines and Standards

❖ Accessibility ICC/ANSI A117.1 1998

Material Properties

Concrete				
Usage	Weight	Strength (psi)		
MAT Slab	Normal	4000psi		
Columns	Normal	4000psi		
Slab on Grade	Normal	3000psi		
Caisson	Normal	4000psi		
Wall	Normal	4000psi		
Grade Beam	Normal	4000psi		
Floor Slab	Normal	4000psi		
Floor Slab	Lightweight	3500psi		
Floor Slab	Normal	3500psi		
Lean Concrete Fill	Normal	2000psi		

Steel				
Туре	Standard	Grade		
Reinforcing Bars	ASTM A615	60		
Composite Floor Deck	ASTM A992	20 gauge		
Roof Deck	ASTM A992	В		
Galvanized Plate	ASTM A992	50		
W shape Steel	ASTM A992	50		
Angles	ASTM A992	50		
Bolts	ASTM A325	N/A		
Anchor Rods	ASTM F1554	N/A		
HSS	ASTM A992	50		
Welded Wire Fabric	ASTM A185	70,000psi		

Masonry			
Туре	Standard	Strength (psi)	
Grout	ASTM C476	5000psi	
Concrete Masonry Units	ASTM C90	2100psi	
Mortar	ASTM C270	N/A	

Miscellaneous			
Type	Strength (psi)		
Non-Shrink Grout	10,000psi		

Figure 4 Tables showing materials that are used in the TCMC project

Foundations

The West wing of the TCMC is built with a mat slab foundation that is 4'-0" thick. The mat slab is designed for a soil bearing pressure of 3000psf. It is on top of a 2'-0" thick structural fill and a 4" mud slab. Figure 5 shows a typical section of the mat slab. After the mat slab, over 4' of compacted AASHTO # 57 stone typical was placed in followed by a 5" slab on grade. Due to the confidentially of the geotechnical report, the actual bearing capacity of the soil and the recommended type of foundations were never released.

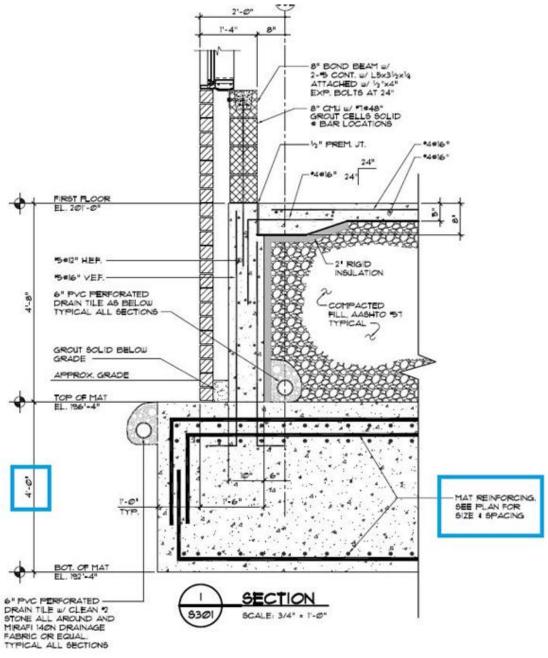


Figure 5 A typical Section cut showing the mat slab foundation. Courtesy of Highland Associates

The East wing of the TCMC has drilled caissons ranging from 36" to 60" in diameter and is used to carry loads from grade beams to bedrock below. The typical floor slab in the east wing is 7.5" and it's also on top of compacted AASHTO material. This can all be visualized by looking at a typical section cut from Figure 6 below.

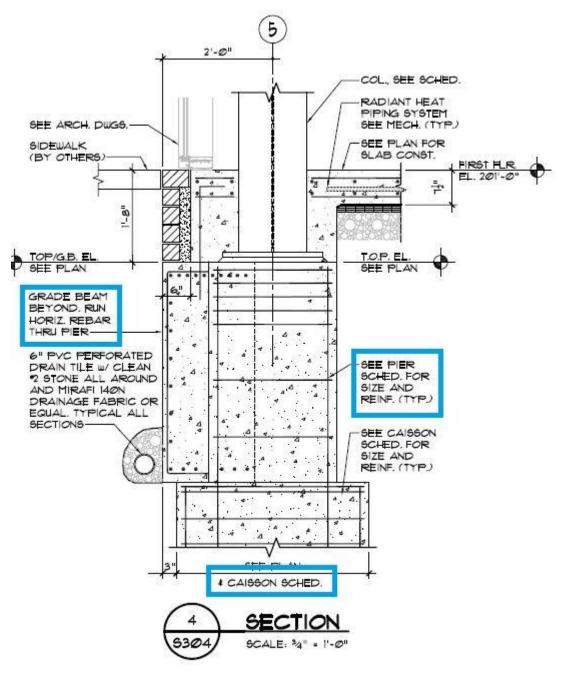


Figure 6 A section cut of a drilled caisson foundation. Courtesy of Highland Associates

Floor Systems

The existing floor system of the TCMC is held up by W-shaped steel columns and composite steel beams. Figure 7 shows the floor plan with different bay sizes in different colors. Bay sizes are shown along with the figure, with the span required for the slab first and the span required for the girder next,

match with their colors. Small bays sizes are not shown in Figure 7.

The floor is composite steel deck with concrete topping. The typical floor plan in the west wing is shown in Figure 8 along with two section cuts, Figures 9 and 10. It is a 4.5" normal weight concrete topping on a 3" lok-floor 20 gauge galvanized composite floor deck, giving it a total slab construction of 7.5". The east wing, and the

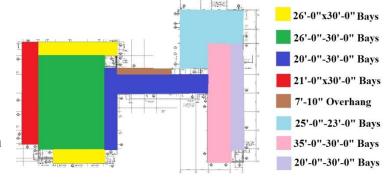


Figure 7 Different Bay sizes respective to their color

link, has different slab thickness than the west wing. They are 3.25" lightweight concrete topping on U.S.D. 2" lok-floor 20 gauge galvanized composite floor deck, making the total thickness of 5.25".

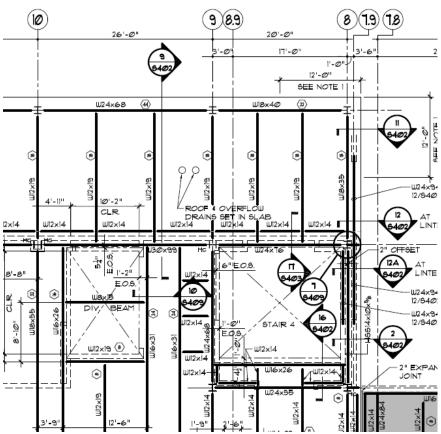


Figure 8 Partial plan showing the second floor, northeast corner of the west wing

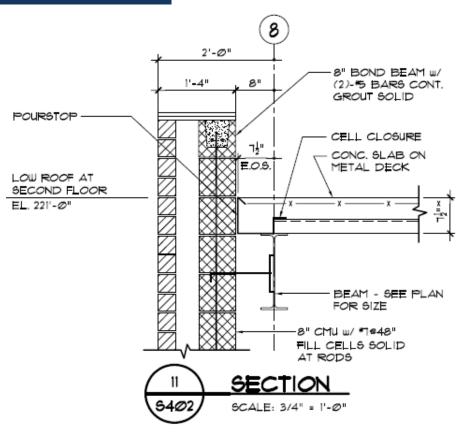


Figure 9 Section cut 11 from Figure 8

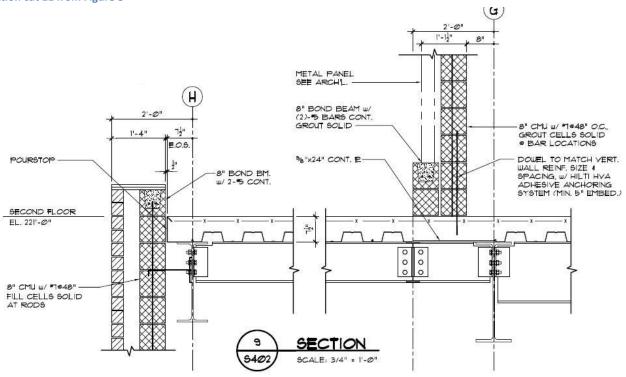


Figure 10 Section cut 9 from Figure 8

Roof Systems

TCMC has over 9 different roof heights, as shown in Figure 11, with the ground referenced at 0'-0". The link between two wings has an average roof height of 36'. The west wing goes up to 92'. The Tower,

shaded in red, in the east wing goes up to 89'-4". The rest of the east wing goes up to 81'-4" while the east wing penthouse goes up to 102'.

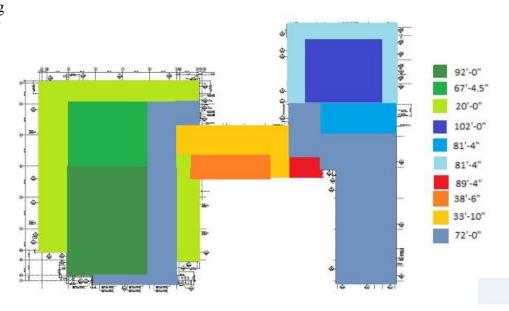


Figure 11 Plan showing the different roof heights; the darker, the higher.

The main roof is constructed of 1.5" type B wide rib, 22 gauge, painted roof deck supported by W-shape framing. A typical roof section cut is shown on Figure 12. The typical roofing system has two layers of 2" rigid roof insulation. The walls around the roof extend 4' higher than the steel deck so that it can be used as railings.

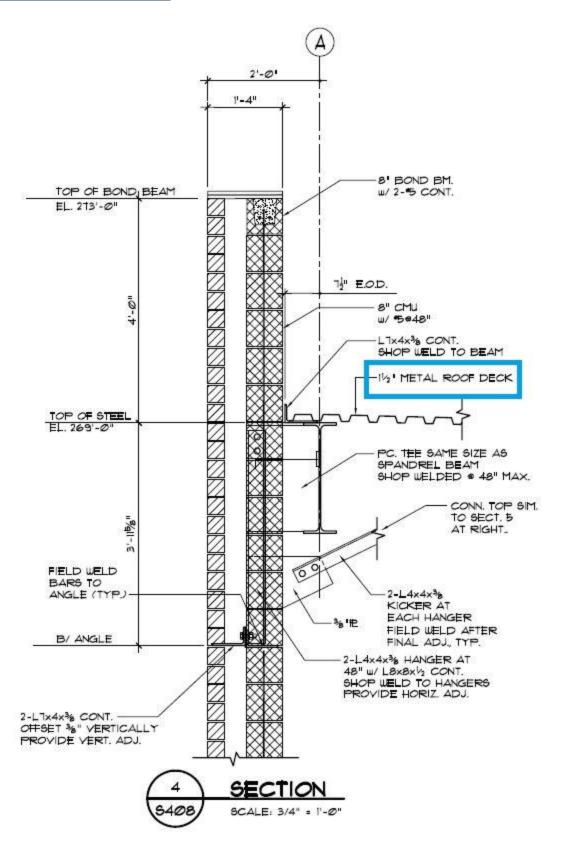


Figure 12 Typical roof section cut showing the roof deck. Courtesy of Highland Associates

Framing System

TCMC has a composite steel framed system. The sizes of the beams and columns ranged from W8x24, being the lightest, to W14x257, being the heaviest. The longest column is 44'-7" and it stopped between the third and fourth floor. An additional 48'-0" of lighter steel column is connected to this column, extending it all the way up to the penthouse.

Lateral System

The main lateral system used in TCMC consists of multiple moment frames. They are present in the west wing, east wing, and also in the link, as shown in Figure 13.1. Most frames are near the exterior wall to maximize the lateral force it can resist. The moment frames span across the entire building, from north to south and from east to west. This provides lateral resistance in each direction. The frames in the link begin on the first floor and extend to the roof, the third floor. The frames in the two wings begin on the first floor and extend to the floor of the penthouse. Figure 13.2 shows the only four frames that extend to the roof of the penthouse.

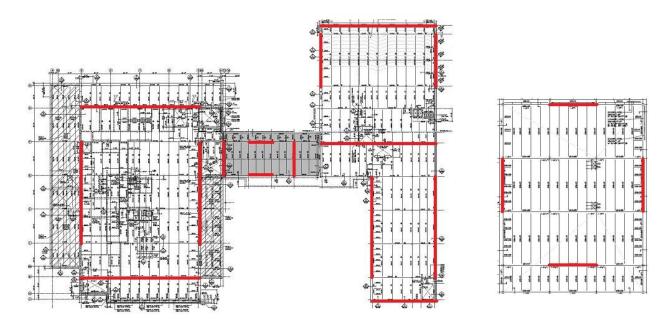


Figure 13.1 Locations of Moment Frames at TCMC. Courtesy of Highland Associates, edited by Xiao Zheng

Figure 13.2 Locations of Moment Frames at the Penthouse of TCMC. Courtesy of Highland Associates, edited by Xiao Zheng

Gravity Loads

The dead, live, and snow loads were calculated under this section for TCMC using IBC 2006, ASCE 7-05, and estimation.

Dead and Live Loads

For the dead load calculations, the materials that have the most impact on the dead weight of the building were found and then calculated. The west wing primarily uses composite 3" steel deck with concrete slab that weighs 75 psf according to Vulcraft Steel Deck catalog. The east wing and the hallway use 2" steel deck, lightweight concrete, so it only weights 42 psf. Then W-shape Steel Beams and Columns are assumed as 15 psf that covers that whole entire building. The heaviest exterior wall is chosen and is assumed throughout the building at 1000plf. Then these weights are multiplied by the area or the length that they occupied in to get the weight in pounds. A sample of this calculation is shown for the 2nd floor of the TCMC in Figure 14 below. Doing this for every level, a weight in psf and lbs are both obtained. Then the total dead weight is found to be around 22,378 kips and will be used later in seismic calculations. A breakdown of the weight per Level is shown in Figure 15.

Weight for 2 nd Floor				
Material	Weight (psf)	Area or Length	Total Weight (lb)	
Normal Weight Conc Slab with Deck	75 (psf)	20408 sf	1,530,600	
Light Weight Conc Slab with Deck	42 (psf)	24952 sf	1,047,984	
W-Shape Steel	15 (psf)	45360 sf	680,400	
Exterior Walls	1000 (plf)	1418 lf	1,418,000	
Total Weigl	4,676,984			
Total Weight per sf (close to design average dead load of 93 psf)			103.11	

Figure 14 Total Weight per square foot of TCMC

Weight Per Level				
Level	Area (ft²)	Weight (psf)	Weight (k)	
1 st	51,348.00	99.3	5099	
2 nd	45,360.00	103.1	4677	
3 rd	40,425.00	106.0	4286	
4 th	40,422.00	106.0	4286	
Penthouse	10,337.00	209.2	2163	
Roof (all level)	40,455.00	46.0	1867	
Total	228,347.00		22378	

Figure 15 Total Weights per Level of TCMC

The design live load for the TCMC can be found in the drawings on sheet S201A and S201B. A comparison of it to the minimum live load requirement from ASCE 7-05 can be seen on Figure 16. Notice that most design load are the same as the minimum required live load. However, some design live loads for several locations are higher because more live loads are expected.

Design Live Loads for West Wing				
Location	Design Live	ASCE 7-05 Live	Notes	
Location	Load (psf)	Load (psf)	Notes	
Offices	50	50		
Lobbies/ Corridors	100	100		
Corridors above 1st	80	80		
Stairs	100	100		
Classrooms	40	40		
Laboratories	100	60	Larger equipment needed in TCMC Labs	
Storage Rooms	125	125	Light warehouse	
Restrooms	60	N/A		
Mechanical Room	150	N/A		
Mechanical Roof	30	N/A		
Roof	20	20	ordinary flat	
Partitions	15	15		

Design Live Loads for Rest of Building			
	Design	ASCE 7-05	
Location	Live	Live	Notes
	Load (psf)	Load (psf)	
Offices above 1st	65	50	Partitions and some heavier office equipment
Lobbies/ Corridors	100	100	
Corridors above 1st	80	80	
Stairs	100	100	
Classrooms	50	40	
Sorage above 1st	125	125	
Restrooms above 1st	75	N/A	
Auditorium	100	100	if seats are fixed, then only 60psf
Bookstore	150	N/A	
Lecture Halls	60	N/A	
Mechanical Room	150	N/A	
Library	75	N/A	
1st floor offices	65	50	
1st floor restrooms	75	N/A	
Roof	30	20	
Mechanical Roof	30	N/A	
1st floor storage	125	100	

Figure 16 Design live load is compared to ASCE 7-05, required live load

Snow Loads

The variables needed for snow load calculations are found on sheet S201B of the drawings. Figure 17 shows all the loads and variables that are from Sheet S201B of the structural drawing. Also, because of the many different roof heights, snow drifts can happen in over 10 different areas of the building. One of these areas is calculated and shown under Appendix A, snow load calculations. The result of that area is that the snow acuminated in the corner reached over 73 psf, more than double the amount compared to the regular flat roof amount of 30 psf. Snow drift is an important factor when designing TCMC.

Flat Roof Snow Load Calculations			
Variable	Value		
Ground Snow Load (Pg)	35 psf		
Flat Roof Snow Load (PF)	30 psf		
Snow Exposure Factor (CE)	1.0		
Importance Factor (Is)	1.1		
Thermal Factor (C⊤)	1.0		

Figure 17 Variable for snow load obtained from S201B

Proposal

Structural Depth

The Commonwealth Medical College is currently being used as a medical college in the middle of a city and was designed for such use. It was designed for the expected wind, seismic and weather loads in Scranton area. For the purpose of this thesis, TCMC will be assumed to be a medical college designed to be built in Miami, Florida, a hurricane prone region. Higher design wind loads from hurricane storms must now be considered. In order to resist the new wind load, the structural members will be redesigned, which will be discussed further later on. As stated before, the existing structure is composed of composite steel framing system. Steel will be used again in this redesign.

Flying debris during a hurricane will cause damage to the façade of the structure. To protect TCMC from such impacts, the façade will be redesigned to withstand the new impact loading requirements.

Not only the weather is different, but the site is also different in Miami and Scranton. The site in Scranton has a much higher bearing capacity, while the site is Miami has a lot less bearing capacity due to its sandy nature. Therefore, a different foundation will be designed.

Proposed Solutions

To meet the new requirements of design for TCMC in Miami, FL, the lateral system will be redesigned along with a new foundation design. The codes that will be used to redesign TCMC in Miami are the Florida Building Code 2010, and ASCE 7-05. Two main systems will be designed and compared in this thesis:

- Moment Frames
- Laterally Braced Frames

Moment frames are the original system for TCMC. It will be redesigned to withstand a larger wind load since it is now in Miami. This frame will be compared to a lateral braced frame system, which is the second system proposed. Both systems will stay with the original gravity system of structural steel. The rest of the gravity system, such as the floor system, will also be changed a little to fit the appropriate new design.

As for the redesign of the façade for impact loading, research of small and large impact resistant design will be done using Florida Building Code 2010 and other sources.

The foundation will be redesigned to account for the different soil condition in Miami Florida. To accomplish this, geotechnical research will be done on a typical site in Miami. In this case, two new foundation designs will be done because the foundation for the two new proposed lateral systems will be slightly different due to the different width of the lateral frames.

Breadth One: Façade Design

By relocating TCMC to Miami, Florida, the climate will be very different from which TCMC was originally designed for. In addition to impact loadings, the proposed new façade redesign will also incorporate heat transfer, waterproofing, as well as day lighting considerations. Breadth 1 focuses on the redesign of TCMC façade to perform better in heat transfer, waterproofing, and day lighting.

Breadth Two: Solar Panel Design

Being a LEED silver certified building, adding solar panels on the roof of TCMC will increase its efficiency of energy usage and possibly increase its LEED rating. Research will be done to confirm this along with the most efficient placement of the solar panels. The climate in Florida will make the photovoltaic system very beneficial because more sunlight can be converted into electricity. Solar panel design will be investigated along with batteries that will be used to store the electricity. This 'free' electric will be used for lighting, emergency system, and other usages.

Breadth Three: Mechanical and Electrical Changes

And the last breath design is on mechanical and electrical changes. The weather conditions in Miami are very different compared to Scranton. Because of this, new mechanical systems need to be analyzed. Most heating units will be replaced by cooling units. The addition of solar panels will impact electrical wirings. The electrical system will be wired so that it can use electricity more efficiently. Lastly, a more powerful emergency backup power will be needed in case of a large hurricane, which is unlikely to happen in Scranton; therefore it is not currently designed for it.

MAE Material Incorporation

The information learned in AE 534, Steel Connections, will be utilized to design a typical braced frame connection and a typical moment frame connection for TCMC. In addition, information learned in AE 537, Building Performance Failures and Forensic Techniques, and AE 542, Building Enclosures, will be used to redesign and detail the façade for impact and pressure resistance, waterproofing, and heat transfer. Lastly, information learned in AE 530, Computer Modeling, will be used to model the appropriate moment frame system and braced frame system.

Tasks and Tools

Structural Depth: Wind Study in Hurricane Prone Region

- I) Determine lateral loads
 - Research Florida Building Code 2010 and ASCE 7-05 to determine the following:
 - Wind loads
 - Hurricane-prone region wind loads
 - Seismic loads
 - Small and large impact loads
- II) Redesign the lateral system for the West Wing, East Wing, and the Link
 - Moment Frames
 - Lateral Braced Frames
 - Model the new lateral systems on ETABS
- III) Foundation design
 - Research if mat slab and drilled caissons foundations can still be used
 - Design a new foundation when TCMC has moment frames
 - Design a new foundation when TCMC has braced frames
- IV) Cost Analysis
 - Use RS Means to find the cost for the two new proposed lateral systems
- V) Comparison
 - Compare the difference between TCMC built in Scranton, TCMC if build in Miami, and TCMC with braced frame build in Miami

Breadth One: Façade Design

- I) AE 542, Building Enclosures- use information learned to help design a new façade
- II) Research the rate of heat transfer in Miami
- III) Find the amount of waterproofing needed
- IV) Research on day lighting accounting for the different angle of the sunlight in Miami, FL.
- V) After research is completed, design the façade in detailed drawings
- VI) Determine the cost difference between new and original façade designs

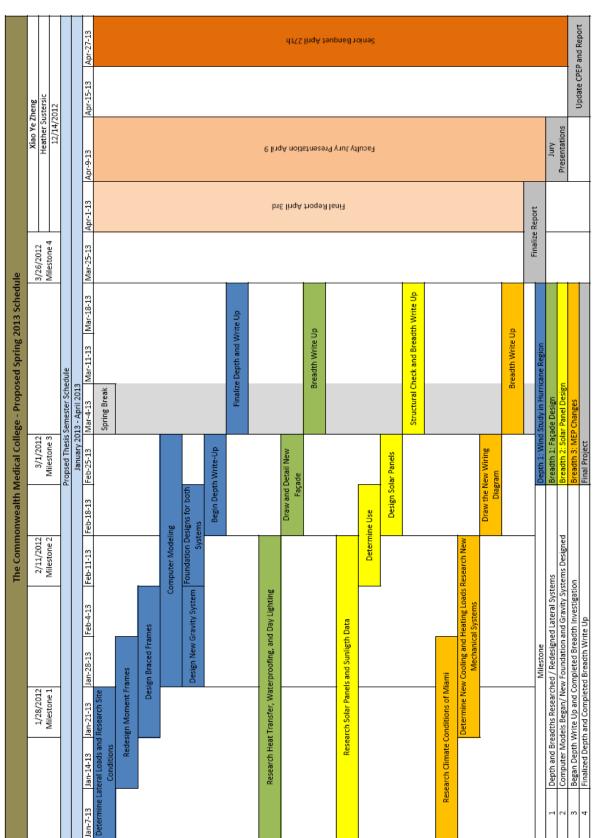
Breadth Two: Solar Panel Design

- I) Determine sunlight data in Miami
- II) Research the best type and the best placement of solar panels for TCMC
- II) Determine standards applicable to solar panels
- III) Determine other buildings in Miami usage of solar panels
- IV) Determine the use of solar panels in TCMC
 - Find the electrical load requirements of TCMC
 - Calculate the size of batteries require to store the energy
 - Determine cost savings and pay off period
- V) Structural Check- Redesign the roof accounting for the new dead loads of the solar panels.
- VI) Determined the cost of the new solar panel design

Breadth Three: Mechanical and Electrical Changes

- I) Electrical Changes
 - Research how to wire the electricity produced from the solar panels back into the building
 - Determined a new backup emergency system: First, find out how much energy is needed
- II) Change the Cooling and Heating Systems
 - Calculate the new loads required for cooling and heating
 - Determine how many heating systems should be removed and how many cooling systems should be added.
- III) Research Cooling Systems
 - Determine which cooling systems is more popular for large buildings in Miami and why it is popular
- IV) Compare cost between new and original Mechanical and Electrical systems

Proposed Schedule



Conclusion

The structural depth for this thesis begins with code research of Miami, Florida. New load have to be determined, along with site conditions and many others. The codes that will be used are the Florida Building Code 2010, and ASCE 7-05. After loads are determined, two new lateral systems will be designed and compared. The first system that will be looked at is the used of moment frames. This is the existing design of TCMC but will be redesigned to meet the new load requirements in Miami. The second system that will be looked at is the steel braced frames. Both situations will implement the original steel gravity system. More importantly, the foundation will have to be redesigned to support the change of lateral systems and the different soil condition.

Three breadth topics will also be included in this thesis. The first breath consists of a new façade design concentrating on heat transfer, waterproofing, and day lighting, along with impact load consideration from the structural depth. The second breath is on solar panel design that could make TCMC more energy efficient by taking advantage of Miami's sunlight. The last breathe in on mechanical and electrical changes because the climate is very different between Scranton and Miami.